DA117/2017 Proposed Residential Aged Care Facility

266 Longueville Road, Lane Cove

REVISED TRAFFIC AND PARKING ASSESSMENT REPORT

28 March 2019

Ref 17311



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Executive Summary

This revised report has been prepared to accompany an amended Development Application to Council (DA117/2017) for a new seniors living development to be located at 266 Longueville Road, Lane Cove.

On Wednesday 11th July 2018, the *Sydney North Planning Panel* meeting was held at Lane Cove Council to discuss a new seniors living development proposal on the subject site, comprising 82 independent living units (ILUs), a 70-bed residential aged care facility (RACF), a two-level basement car parking area for 122 cars, plus a loading bay and ambulance bay.

Due to a number of concerns raised by local residents the Panel chose to defer the matter, subject to additional information being provided by the Applicant, including obtaining an independent peer review of the traffic and parking assessment report prepared by *Varga Traffic Planning (VTP)*.

Council therefore engaged *Bitzios Consulting (Bitzios)* to undertake the peer review of the traffic study who recommended that a number of modifications were required of the SIDRA traffic model, including taking into account the RMS's concept signal plan for realignment works at the River Road West & Longueville Road intersection, allowing right turn movements from Longueville Road onto River Road West for *all* traffic.

It is pertinent to note that *VTP* only received a copy of the RMS's concept signal plan the afternoon of the Panel meeting, such that the *VTP* traffic model was based on the abovementioned right turn movements from Longueville Road onto River Road West restricted to *buses only*.

Accordingly, the VTP SIDRA traffic model has been revised to take into account the *Bitzios* peer review comments, including allowing right turn movements from Longueville Road onto River Road West for *all* traffic. It is also worth noting that whilst the *Bitzios* peer review traffic study provided a number of recommendations, no SIDRA files or results were provided to VTP for cross-scrutiny.

In essence, the revised capacity analysis reconfirms that the traffic generation potential of the development proposal on the subject site will not result in the reduction in the *Level of Service* of the nearby intersections.

Other modifications to the design, from a traffic and parking perspective, include a new 2m wide landscaping strip along the southern boundary of the site to improve privacy to the adjacent "Timber Tops" residential development, as recommended by the Panel. In doing so, the site access driveway has shifted 2m to the north.

In addition to the *Bitzios* peer review which was commissioned by Council, "Timbertops" residents engaged *ML Traffic* (*ML*) to also undertake a peer review of the *VTP* report. Notwithstanding, the *ML* peer review was based on an earlier version of the *VTP* report and therefore outdated information, as well as questionable and incorrect assumptions on certain matters.

In summary, the amended Development Application will not result in any unacceptable traffic implications and complies with the applicable numerical off-street parking requirements.

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1. INTRODUCTION

This revised report has been prepared on behalf of *Australian Unity* to accompany an amended Development Application to Council for a seniors living development to be located at 266 Longueville Road, Lane Cove (Figures 1 and 2).

The proposed development involves the demolition of the former lawn bowls facilities and associated car parking area on the site to facilitate the construction of a new seniors living development, comprising a 70-bed residential aged care facility as well as 82 independent living units.

Off-street car parking is to be provided for a total of 122 spaces, plus an ambulance bay, in a new two-level basement parking area in accordance with *State Environmental Planning Policy (Housing for Seniors or People with a Disability) 2004* and Council's requirements.

The purpose of this revised report is to assess the traffic and parking implications of the amended development proposal and to that end this report:

- describes the site and provides details of the development proposal
- reviews the road network in the vicinity of the site
- reviews the public transport services available in the vicinity of the site
- estimates the traffic generation potential of the development proposal
- assesses the traffic implications of the development proposal in terms of road network capacity
- reviews the geometric design features of the proposed car parking and loading facilities for compliance with the relevant codes and standards
- assesses the adequacy and suitability of the quantum of off-street car parking and loading provided on the site.





2. PROPOSED DEVELOPMENT

Site

The subject site is located on the eastern side of Longueville Road, approximately 100m south of Richardson St West. The site has a street frontage approximately 101m in length to Longueville Road and occupies an area of approximately 9,200m².

The subject site was previously occupied by two lawn bowls greens, a club house building and an associated car parking area. The bowling greens are no longer used, and the existing building on the site is currently used to accommodate the Lane Cove Music and Cultural Centre. A recent aerial image of the site and its surroundings is reproduced below.



Car parking is currently provided in two separate areas on the site, comprising a small informal car parking area located to the rear of the existing building on the site, and a larger car parking area at the north-western corner of the site which is used primarily for boat trailer storage.

Vehicular access to the site is provided via two separate driveways which are located adjacent to the southern boundary of the site and also midway along the site frontage, further to the north in Longueville Road.

The southern vehicular access driveway in Longueville Road is shared with the adjacent residential apartment building known as "Timber Tops" which is located immediately to the south of the subject site.

Proposed Development

The proposed development involves the demolition of the former lawn bowls facility and associated car parking area on the site to facilitate the construction of a new seniors living development.

A total of 82 independent living units (ILU's) are proposed as follows:

TOTAL APARTMENTS:	82
2 bedroom apartments:	71
1 bedroom apartments:	11

A total of 70 residential aged care (RACF) beds are also proposed within the new building.

There will be a maximum of 25 staff during the main day shift, with 34 staff on-site during the afternoon shift change. In this regard, the afternoon shift change occurs between 2:45pm-3:00pm however staff often arrive up to 30 minutes *earlier* whilst staff often leave up to 30 minutes *later* (typical of many workplaces) – i.e. not all staff will arrive and depart the site during the abovementioned 15 minute "window", in reality those movements would likely be distributed across a full *hour* period.

A number of ancillary facilities are proposed for the use of residents, including several common areas such as dining rooms, TV lounges and a courtyard. A number of back-of-house areas are also proposed, such as an administration/community facilities office, laundry, kitchen, staff lockers and a maintenance room.

Off-street parking is proposed for a total of 122 cars, plus an ambulance bay, in a new twolevel basement car parking area in accordance with *SEPP 2004* and Council's requirements. The amended scheme now also makes provision for ILU visitor parking within the basement.

A porte-cochere is also proposed to facilitate the drop-off and pick-up of (mostly elderly) passengers, with entry via a one-way driveway located midway along the site frontage and exit via the main site access driveway.

Vehicular access to the site is to be provided a new entry/exit driveway which is to be located towards the southern end of the Longueville Road site frontage, near the *existing* driveway which currently provides vehicular access to the adjacent "Timber Tops" residential development. In this regard, the proposed new driveway has been shifted 2m to the north, thereby creating a 2m wide landscaping strip along the southern boundary of the site for improved privacy between the two developments – i.e. the subject development and "Timber Tops".

The proposed new driveway will also provide vehicular access to the "Timber Tops" development by way of a registered easement. The driveway levels have been designed to comply with *AS2890.2* as well as tie into the existing levels in the vicinity of the "Timber Tops" car parking area, approximately 30m from the front boundary, ensuring that vehicular access to the "Timber Tops" residential development is not disrupted.

Furthermore, the main access driveway has been modified to include two splayed driveways separated by a raised kerb and concrete median *within* the property boundary in order to restrict turning movements to left-in/left-out only, generally in accordance with RMS's standard design.

It is pertinent to note that the location of the proposed site access driveway in Longueville Road complies with Council's site specific *DCP 2010, Part C, Residential Localities – Locality 7 – 266 Longueville Road (Amendment 25 March 2015)* document which requires access to be provided via the existing easement – i.e. via the "Timber Tops" driveway. During the preparation of the site-specific *Draft DCP*, consultation was made with the RMS who provided their support of the driveway location which was then adopted into the amended *DCP*. In addition, the Panel note that any driveway located at the northern end of the Longueville Road site frontage "would result in an increase in the impact on the neighbours to the north, increase the amount of paving on the site and create two access points instead of one".

The servicing and delivery needs of the proposed development are expected to be undertaken by light commercial vehicles such as "white vans" and the like and small trucks up to and including 6.4m long small rigid trucks. All garbage collection services are to be undertaken by private contractors using a small rigid truck. A servicing/delivery area is proposed at the eastern end of the internal driveway/ramp to accommodate these vehicles, such that they will be able to enter and exit the site whilst travelling in a forward direction at all times.

Proposed Intersection Upgrade

Since the lodgement of the original Development Application, Council and the RMS have explored options to upgrade the intersection of River Road West & Longueville Road. The concept signal plans are reproduced in Appendix A and summarised on the following page.

The short-term option includes:

- provision of a designated right turn lane from Longueville Road into River Road West; and
- redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road

The long-term option includes:

- provision of a designated right turn lane from Longueville Road into River Road West; and
- provision of a dedicated right turn lane into Longueville Road and Kenneth Street; and
- redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road.

RMS also note that subject to funding, approval has been granted for the short-term option. Funding for the long-term option is unknown at this stage. As recommended in the *Bitzios* peer review traffic study, the future scenario - i.e. existing development *plus* development traffic - is based on the proposed new short-term intersection layout.

Plans of the proposed amended development have been prepared by *Thomson Adsett Architects* and are reproduced in the following pages.

















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3. TRAFFIC ASSESSMENT

Road Hierarchy

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services is illustrated on Figure 3.

Epping Road is classified by the RMS as a *State Road* and provides the key east-west road link in the area, linking Epping to Lane Cove. It typically carries two traffic lanes in each direction in the vicinity of the site (including a 24-Hour Bus Lane in both directions), with additional lanes provided at key locations.

River Road is classified by the RMS as a *Regional Road* and provides the key east-west road link in the area. It typically carries 4 traffic lanes (ie; two lanes in each direction), with some kerbside parking permitted in selected locations only, outside peak periods.

Longueville Road is a local, unclassified road which performs the function of a *Collector Route*. It typically carries one traffic lane in each direction, with kerbside parking generally permitted on both sides of the road.

Existing Traffic Controls

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

- a 50 km/h SPEED LIMIT which applies to all roads in the surrounding area
- a 40 km/h SCHOOL ZONE SPEED LIMIT which applies to Longueville Road, within the vicinity of Currambena Primary School and St Michael's Catholic Primary School
- TRAFFIC SIGNALS in River Road where it intersects with Longueville Road
- TRAFFIC SIGNALS in Northwood Road where it intersects with Kenneth Street





• NO RIGHT-TURN (Buses Excepted) restriction for southbound traffic in Longueville Road at its intersection with River Road West.

Existing Public Transport Services

The existing public transport services available to the site are illustrated on Figure 5. There are currently three bus routes travelling along Longueville Rd past the site, comprising regular services to North Sydney and the City, as well as a number of peak hour express services along the Gore Hill Freeway to/from the City.

There are more than 110 bus services travelling past the site on weekdays (with a peak hour frequency of 8 services per hour), decreasing to 60 bus services per day on Saturdays and 30 services per day on Sunday and public holidays, as set out in the table below:

	Bus Routes and Frequencies						
Route	Darata	Weekdays Sa		Satu	rday	Sunday	
No.	Koute	IN	OUT	IN	OUT	IN	OUT
253	Riverview to City (via Fwy)	7	5	-	-	-	-
254	Riverview to City (via Fwy)	29	25	20	19	15	15
261	City King St Wharf to	23	25	11	12	-	-
	Chatswood via Longueville						
TOTAL		59	55	31	31	15	15

All of the abovementioned bus services traverse the Lane Cove shops where an extensive range of shops (including supermarkets), restaurants, cafes and services such as banks and the post office are located.

All of the bus services also traverse the Bus Interchange located on Longueville Road (near the intersection of Epping Rd), allowing interchange with other connecting bus services to Epping, Chatswood, St Leonards railway stations and the medical facilities located in the vicinity of RNS Hospital.

In addition, an interchange is also possible on the Pacific Hwy near Gore Hill TAFE college with the intra-regional Metrobus M20 service which operates between Artarmon and Botany via Central Railway seven days per week, with weekday services every 15 minutes (every 10 minutes during the morning and afternoon peak) and weekend services every 20 minutes.



The site is therefore considered to be ready accessible by public transport services. Experience at other *Australian Unity* RACFs indicate that a substantial portion of staff travel to/from work via public transport.

Existing Traffic Conditions

An indication of the existing traffic conditions on the road network in the vicinity of the site is provided by peak period traffic surveys undertaken as part of this revised traffic study. The traffic surveys were undertaken at the Longueville Road & River Road intersection, the Northwood Road & Kenneth Street intersection and also the existing "Timber Tops" site access driveway. The results of the traffic surveys are reproduced in full in Appendix B and reveal that:

- the morning network peak period occurred between 8:00am and 9:00am whilst the afternoon network peak period occurred between 4:45pm and 5:45pm
- two-way traffic flows past the site frontage in Longueville Road on the day of the surveys were in the order of 700 vehicles per hour (vph) during the morning and afternoon peak period, and
- two-way traffic flows using the existing "Timber Tops" site access driveway were 6 vph during the morning peak period and 8 vph during the afternoon peak period.

Projected Traffic Generation

An indication of the traffic generation potential of the proposed development is provided by reference to the Roads and Maritime Services publication *Guide to Traffic Generating Developments, Section 3 – Land Use Traffic Generation (October 2002)* and the updated *Technical Direction TDT 2013/04a*.

The RMS *Guidelines* and the *TDT 2013/04a* are based on extensive surveys of a wide range of landuses and nominate the following traffic generation rates which are applicable to the development proposal:

Housing Seniors

0.4 peak hour vehicle trips per dwelling

The RMS *Guidelines* also make the following observation in respect of housing for aged and disabled persons:

Definition

Residential accommodation which may take any building form which is to be intended to be used permanently as housing for the accommodation of aged or disabled persons. The hostel may consist of residencies or a grouping of 2 or more self-contained dwellings and include facilities such as staff accommodation, chapels, medical rooms, recreation facilities, shops and/or therapy rooms.

Factors

These figures at the lower end of the above rates concentrate on *subsidised* developments (often run by religious organisations). Generation rates of *resident fun*ded developments are often greater, as indicated at the higher end of the range.

Application of the above traffic generation rate to the 82 ILUs and 70 standard aged care beds outlined in the development proposal yields a traffic generation potential of approximately 61 vehicle trips per hour during commuter peak periods as follows:

PROJECTED TRAFFIC GENERATION	
82 Independent Living Units:	33 vph
70 Aged Care Beds:	28 vph
TOTAL TRAFFIC GENERATION POTENTIAL:	61 vph

In practice, the actual traffic generation potential of the proposed development is likely to be *less* than is set out in the table above, as RACF beds tend to generate *less traffic* activity than ILUs. The RMS *TDT 2013/04a* also makes the following observations:

"Note that morning site peak hour does not generally coincide with the network peak hour".

Furthermore, the RACF site peak will also *not* coincide with the network peak period as the morning shift change will occur *prior to* the morning network peak which is 8:00am and 9:00am whilst the afternoon shift change will occur *prior to* the afternoon network peak which is 4:45pm and 5:45pm.

Peak visitor periods for both the ILUs and RACF will likely be during the middle of the day and on weekends when on-road traffic is much lower than the morning and afternoon peak periods.

In reality, the only component of the proposed development that *might* occur during the network peak periods is the ILUs in the *afternoon* which is 4:45pm and 5:45pm.

Whilst the *Bitzios* peer review traffic study recommended that a *nett traffic generation* using 53 AM trips and 53 PM trips be adopted (i.e. proposed less existing), this recommendation has *not* been implemented in order to provide a more *rigorous* assessment, therefore the above traffic generation potential of 61 vph has continued to be adopted to assess the traffic impacts on the adjacent public road network, as set out below. Furthermore, the SIDRA model also takes into account the RMS request for all turning movements into/out of the site to be restricted to left-in/left-out only.

In any event, that projected increase in traffic activity as a consequence of the development proposal is minimal and will clearly not have any unacceptable traffic implications in terms of road network capacity, as is demonstrated by the following section of this report.

Traffic Splits & Trip Distribution

The table below indicates the traffic splits that have been adopted in the traffic model.

Development	AM Peak Volumes		PM Peak	Volumes
	In	Out	In	Out
Subject Site	30 vph	31 vph	31 vph	30 vph
Timber Tops	0 vph	6 vph	5 vph	3 vph
TOTAL	30 vph	37 vph	36 vph	33 vph

In terms of distribution, it has been assumed that development will approach and depart the site *evenly distributed* between the west via River Road West, the north via Longueville Road and the east via Northwood Road.

It is worth noting that the *Bitzios* peer review traffic study adopted a distribution of 20% to the west via River Road West, 30% to the north via Longueville Road, 40% to the east along Northwood Road and 10% to the south via William Edward Street.

Direction	AM Peak Volumes		PM Peak	Volumes
	In	Out	In	Out
North	10 vph	12 vph	12 vph	11 vph
South	0 vph	0 vph	0 vph	0 vph
East	10 vph	13 vph	12 vph	11 vph
West	10 vph	12 vph	12 vph	11 vph

Whilst the distributions adopted in the traffic model differ slightly to the *Bitzios* distributions, in traffic volume terms there are minimal differences between the two.

At present, one option for any vehicle, other than buses, wishing to head west along River Road West after travelling south along Longueville Road would be to cross over Northwood Road onto Kenneth Street, then west onto New Street, north onto William Edward Street and then west along River Road West, as indicated in the figure below. This would also be the case for the proposed development's traffic when exiting the site.



Proposed Traffic Distribution - Existing Arrangements

In accordance with the recommendations in the *Bitzios* peer review traffic study, the traffic model takes into consideration the proposed new short-term upgrade of the River Road West & Longueville Road traffic signals which will permit right turn movements for traffic turning from Longueville Road onto River Road West for *all* traffic, as indicated in the figure below.



Proposed Traffic Distribution - Proposed Arrangements

For the purposes of this assessment it has been assumed that 10% of the existing traffic turning right into Kenneth Street from Northwood Road is performing the abovementioned circuit back to River Road West (i.e. 16 vph AM and 19 vph PM). These volumes have therefore been removed from the relevant turning volumes and added to the proposed new right turn directly onto River Road West.

The proposed new upgrade of the River Road West and Longueville Road traffic signals which will allow right turn movements for *all* vehicles, not just buses, will therefore remove in the order of 40 vph from the Kenneth Street/New Street/William Edward Street circuit.

SIDRA Traffic Model

The traffic impacts of development proposals primarily concern the effects that the additional traffic flows the development generates may have on the operational performance of the nearby road network. Those effects can be assessed using the SIDRA NETWORK capacity analysis program as is widely used by the RMS and many LGAs for this purpose. Criteria for evaluating the results of SIDRA NETWORK capacity analysis are reproduced in the following pages.

The *Bitzios* peer review traffic study recommended that a number of modifications were required to the traffic model including intersection geometry, lane widths, network geometry, priorities, phasing and timing and also based on the RMS's IDM (Independent Diagnostic Monitor) data. A full copy of the IDM data is reproduced in Appendix C. Screenshots of the intersection layouts of the IDM data are reproduced below.



Source: RMS

Traffic Implications - Road Network Capacity

The results of the revised SIDRA capacity analysis, which take into consideration the *Bitzios* recommendations, are reproduced in full in Appendix D and summarised in the following pages. The SIDRA results of the nearby Longueville Road/River Road intersection are summarised in the following tables.

TABLE 1 – RESULT OF SIDRA ANALYSIS LONGUEVILLE ROAD & RIVER ROAD				
EXISTING PROPOSED				
	AM	PM	AM	PM
Level of Service	E	В	С	В
Average Vehicle Delays	67.3	16.5	28.8	21.3
Degree of Saturation	1.030	0.630	0.861	0.711

TABLE 2 – RESULT OF SIDRA ANALYSIS NORTHWOOD ROAD & KENNETH STREET				
	EXISTING PROPOSED			
	AM	PM	AM	PM
Level of Service	В	В	В	В
Average Vehicle Delays	16.1	23.9	17.3	21.4
Degree of Saturation	0.922	0.895	0.938	0.866

TABLE 3– RESULT OF SIDRA ANALYSIS LONGUEVILLE ROAD & SITE ACCESS DRIVEWAY				
	EXISTING PROPOSED			
	AM	PM	AM	PM
Level of Service	А	А	А	А
Average Vehicle Delays	0.0	0.1	0.2	0.2
Degree of Saturation	0.245	0.197	0.244	0.195

In summary therefore, the revised capacity analysis reconfirms that the traffic impacts of the development proposal will be *statistically insignificant* and will clearly not have any unacceptable traffic implications in terms of road network capacity.

Criteria for Interpreting Results of Sidra Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
Έ'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
А	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

4. PARKING IMPLICATIONS

Existing Kerbside Parking Restrictions

The existing kerbside parking restrictions which apply to the road network in the vicinity of the site are illustrated on Figure 6 and comprise:

- NO STOPPING restrictions in the vicinity of the Longueville Road/River Road West intersection
- BUS ZONES located on both sides of Longueville Road at regular intervals, including directly in front of the site
- NO PARKING restrictions on the eastern side of Longueville Road between the southern boundary of the site and the traffic signals at the River Road West intersection
- UNRESTRICTED KERBSIDE PARKING elsewhere along this section of Longueville Road.

Off-Street Parking Provisions

The off-street parking requirements applicable to the development proposal are specified in Council's *DCP 2006* and in the *State Environmental Planning Policy (Housing for Seniors or People with a Disability) 2004* which specify the following off-street car parking requirements applicable to the development proposal:

Division 4 Self Contained Dwellings

50 Standards that cannot be used to refuse development consent for self-contained dwellings

A consent authority must not refuse consent to a development application made pursuant to this Chapter for the carrying out of development for the purpose of a self-contained dwelling (including in-fill self-care housing and services self-care housing) on any of the following grounds:

(d) parking: if at least the following is provided:



- (i) 0.5 parking space for each bedroom where the development application is made by a person other than a social housing provider
- (ii) 1 car space for each 5 dwellings where the development application is made by, or is made by persons jointly with a social housing provider.

Division 2 Residential Care Facilities

48 Standards that cannot be used to refuse development consent for residential care facilities

A consent authority must not refuse consent to a development application made pursuant to this Chapter for the carrying out of development for the purpose of a residential care facility on any of the following grounds:

- (d) parking for residents and visitors: if at least the following is provided:
 - (i) 1 parking space for each 10 beds in the residential care facility (or 1 parking space for each 15 beds if the facility provides care only for persons with dementia), and
 - (ii) 1 parking space for each 2 persons to be employed in connection with the development and on duty at any one time, and
 - (iii) 1 parking space suitable for an ambulance.

Application of the above parking requirements to the development proposal yields an offstreet car parking requirement of 101 spaces as set out in the table below:

TOTAL:	100.5 spaces
Residential Aged Care Staff (max. 34 staff at PM changeover):	17.0 spaces
Residential Aged Care Visitors (70 beds - standard):	7.0 spaces
Independent Seniors Living Units: (82 dwellings):	76.5 spaces

Whilst the *SEPP* does not nominate an off-street parking rate for visitors to the ILUs, visitor parking in the amended scheme is now proposed at a rate of *1 space per 4 units* – i.e. 21 spaces – consistent with the visitor parking rate for residential flat buildings nominated in Council's *DCP*.

The proposed development makes provision for a total of 122 off-street parking spaces, plus an ambulance bay, thereby satisfying the *SEPP* (and Council's *DCP*) requirements.

The geometric design layout of the proposed car parking facilities has been designed to comply with the relevant requirements specified in the Standards Australia publication *Parking Facilities Part 1 - Off-Street Car Parking AS2890.1 - 2004* in respect of parking bay dimensions and aisle widths.

The amended vehicular access arrangements have also been designed to accommodate the largest vehicle expected to service the site which is a 6.4m long SRV truck. A series of swept turning path diagrams have been prepared which demonstrate that SRV trucks, and light passenger vehicles, can enter and exit the site in a forward direction at all times.

In summary, the proposed parking, loading and access arrangements satisfy the relevant requirements specified in *SEPP (2004)*, Council's *DCP*, Australian Standards and by the RMS, and it is therefore concluded that the proposed development will not have any unacceptable parking, servicing or access implications.






APPENDIX A

CONCEPT TRAFFIC SIGNAL PLAN

10 JULY 2018

10 July 2018

The General Manager Lane Cove Municipal Council PO Box 20 LANE COVE NSW 1595

Attention: Sashika Perera, Coordinator - Traffic and Transport

Re: River Road West/Longueville Road Intersection Upgrade

Roads and Maritime Services have reviewed the River Road West/Longueville Road intersection and support Council with its preferred short-term and long-term options.

The short-term option includes:

- Provision of a designated right turn lane from Longueville Road into River Road West; and
- Redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road.

The long-term option includes:

- Provision of a designated right turn lane from Longueville Road into River Road West; and
- Provision of a dedicated right turn lane into Longueville Road and Kenneth Street; and
- Redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road.

It is noted that subject to funding, approval has been granted for the implementation of the short-term option.

Yours sincerely

Vicky Walker, Senior Network and Safety Officer North Roads and Maritime Services

Roads and Maritime Services

27-31 Argyle Street, Parramatta NSW 2150 | PO Box 973, Parramatta, NSW 2124 |

www.rms.nsw.gov.au | 13 22 13

CO-ORDINATE SYSTEM MGA ZONE 56

HEIGHT DATUM

AHD

J. SALES-LUIS XXXXXXX DESIGN CHECK XUXXXX NETWORK AND SAFETY SERVICES XXXXXX NORTH-WEST PRECINCT DESIGN MNGR J. SALES-LUIS PROJECT MNGR V. WALKER

ALIGNMENT DIRECTS TRAFFIC INTO THE LEFT TURN AND EXAGGERATES THE RIGHT TURN INCREASING AWARENESS

DISTANCE BETWEEN SIGNALS INCREASED SLIGHTLY NUMBER OF PED CROSSING REQUIRED IS REDUCED COMPARED WITH OPTION RD02

DISADVANTAGES ALIGNMENT AT SIGNALS AT KENNETH ST REMAINS CONFUSING FOR DRIVERS - LACK OF ADVANCED WARNING REMAINS CONFUSING FOR

- DEDICATED RIGHT TURN LANE NOT PROVIDED AND DOES NOT REDUCE RISK OF LANE CHANGE INCIDENTS - LONGUEVILLE RD REDUCED TO ONE LANE INTO NORTHWOOD RD (UNLIKELY TO CAUSE CONGESTION - LEFT TURNING TRAFFIC FROM LONGUEVILLE RD NO LONGER HAS DIRECT PATH ONTO NORTHWOOD RD OPTION REQUIRES TRAFFIC MODELLING

NORTHWOOD ROAD

3.0

3.2

FROM CROWS NEST

COMMERCIAL

RECOMMENDED TO INSTALL DIAGRAMMATIC GUIDE SIGN (SEE DRAWING AP01)

TO LONGUEVILLE KENNETH STREET

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	10.1-1-1	NOT F	OR CONS	TRUCTI	ON
ne	LANE COVE COUNCIL RR2070 - RIVER ROAD INTERSECTION I NORTHWOOD RI LONGUEVILLE OPTION RD02-1	AREA WEST MPROVEN D AND LOI	IENTS AT NGUEVILLE RI	D SHEET 3 0	A3
	RMS REGISTRATION No. DS	52018/	000623		PART
	ISSUE STATUS STRATEGIC DESIGN		EDMS No. SF2018 / 183121	RD02-1	ISSUE
			© Roads a	and Maritime S	Service

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ADVANTAGES - ALIGNMENT DIRECTS TRAFFIC INTO THE LEFT TURN AND EXAGGERATES THE RIGHT TURN INCREASING AWARENESS - DISTANCE BETWEEN SIGNALS INCREASED SLIGHTLY IMPROVED ALIGNMENT FROM KENNETH STREET TO LONGUEVILLE RD - DEDICATED RIGHT TURN LANE NORTHBOUND DECREASES LANE CHANGE ACCIDENTS - RIGHT TURN INTO KENNETH STREET MAINTAINED DISADVANTAGES - LONGUEVILLE RD REDUCED TO ONE LANE INTO NORTHWOOD RD (UNLIKELY TO CAUSE CONGESTION HOWEVER TRAFFIC MODELLING SHOULD BE UNDERTAKEN) - RIGHT TURN LANE INTO LONGUEVILLE RD MAY NOT BE LONG ENOUGH TO CATER FOR DEMAND, SHORT SIGNAL PHASES COULD ASSIST THIS - REQUIRES ACQUISITION INTO BOWLING CLUB AND PETROL STATION - OPTION REQUIRES TRAFFIC MODELLING NORTHWOOD ROAD FROM CROWS NEST 3.0 2 3 Nº PAR 37.1.200 COMMERCIAL RECOMMENDED TO INSTALL DIAGRAMMATIC GUIDE SIGN (SEE DRAWING AP01) TOLONGUEVILLE KENNETH STREET NOT FOR CONSTRUCTION LANE COVE COUNCIL AREA A3 RR2070 - RIVER ROAD WEST INTERSECTION IMPROVEMENTS AT NORTHWOOD RD AND LONGUEVILLE RD LONGUEVILLE OPTION RD07 SHEET 8 OF 9 RAIS REGISTRATION NO. DS2018 / 000623 ISSUE STATUS ISSUE EDMS No. SF2018 / 183121 RD07 STRATEGIC DESIGN © Roads and Maritime Services

APPENDIX B

TRAFFIC SURVEY DATA

	R.O	.A.R	DA	ТА																			
	Relia	eliable, Original & Authentic Results		S	PEDS	NO	RTH	E/	AST	SO	UTH		PEDS	NO	RTH	EA	ST	SO	UTH				
	Ph.88	196847.	Mob 04	182390	019			Time Per	Long	ueville	Ac	cess	Long	ıeville	тот	Peak Per	Long	ueville	Acc	ess	Longu	ıeville	тот
								0630 - 0645		0		3		0	3	0630 - 0730		1	1	7		2	20
								0645 - 0700		0		5		0	5	0645 - 0745		1	2	20		4	25
Clien	t	: Varg	a Traff	ic Plan	ning			0700 - 0715		0		5		0	5	0700 - 0800		2	1	7	(6	25
Job No/N	lame	: 6772	2 LANE	COVE	E Long	ueville	Rd	0715 - 0730		1		4		2	7	0715 - 0815		2	2	21	(6	29
Day/Da	ate	: Tues	sday 1s	st May	2018			0730 - 0745		0		6	:	2	8	0730 - 0830		2	2	28	4	4	34
								0745 - 0800		1		2	:	2	5	0745 - 0845		2	2	27	1	2	31
								0800 - 0815		0		9		0	9	0800 - 0900		2	2	27		1	30
								0815 - 0830		1		11		0	12	0815 - 0915		3	2	25	2	2	30
								0830 - 0845		0		5		0	5	0830 - 0930		3	1	6	:	3	22
								0845 - 0900		1		2		1	4								
								0900 - 0915		1		7		1	9	PEAK HR		2	2	27	-	1	30
								0915 - 0930		1		2		1	4								
								Per End		6	(61		9	76								
Lights	NO	ртц	=	ет	50			Hoavies	NO	ртц	E	AST	50			Combined	NO	ртц	=	ет	50	пц	
Lights	Long	ueville		101	Long	ueville		rieavies	Long	ueville		431	Long	Jeville		combined	Long	ueville			Longu	ieville	
	F	Rd	Acc	ess	Ē	Rd			F	Rd	AC	cess	Ē	d			F	d	ACC	ess	R	d	
Time Per	T	L	<u>R</u>	L	<u>R</u>	<u>T</u>	тот	Time Per	T	L	<u>R</u>	L	<u>R</u>	<u>T</u>	тот	Time Per	<u>T</u>	L	<u>R</u>	L	<u>R</u>	<u>T</u>	тот
0630 - 0645	44	0	0	0	0	38	82	0630 - 0645	1	0	0	0	0	2	3	0630 - 0645	45	0	0	0	0	40	85
0645 - 0700	37	0	1	0	0	74	112	0645 - 0700	4	0	0	0	0	1	5	0645 - 0700	41	0	1	0	0	75	117
0700 - 0715	64	0	0	0	0	65	129	0700 - 0715	5	0	0	0	0	3	8	0700 - 0715	69	0	0	0	0	68	137
0715 - 0730	39	0	0	0	0	102	141	0715 - 0730	5	0	0	0	0	2	7	0715 - 0730	44	0	0	0	0	104	148
0730 - 0745	48	0	0	0	1	82	131	0730 - 0745	2	0	0	0	0	6	8	0730 - 0745	50	0	0	0	1	88	139
0745 - 0800	39	0	1	0	0	93	133	0745 - 0800	1	0	0	0	0	2	3	0745 - 0800	40	0	1	0	0	95	136
0800 - 0815	53	0	1	1	0	86	141	0800 - 0815	1	0	0	0	0	4	5	0800 - 0815	54	0	1	1	0	90	146
0815 - 0830	67	0	1	1	0	123	192	0815 - 0830	2	0	0	0	0	4	6	0815 - 0830	69	0	1	1	0	127	198
0830 - 0845	59	0	1	0	0	139	199	0830 - 0845	1	0	0	0	0	3	4	0830 - 0845	60	0	1	0	0	142	203
0845 - 0900	86	0	0	1	0	98	185	0845 - 0900	1	0	0	0	0	3	4	0845 - 0900	87	0	0	1	0	101	189
0900 - 0915	48	0	0	0	0	82	130	0900 - 0915	3	0	0	0	0	2	5	0900 - 0915	51	0	0	0	0	84 67	135
0915 - 0930	4Z	0	5	2	2	1049	169/	0915-0930	ు 20	0	0	0	0	22	4 62	0915 - 0930	40 655	0	5	0	2	1091	1746
	020	U	3	3	2	1040	1004		ZJ	U	U	U	U	- 33	02		033	U	3	3	2	1001	1740
Lights	NO	RTH	EA	ST	SO	UTH		<u>Heavies</u>	NO	RTH	E/	AST	SO	UTH		<u>Combined</u>	NO	RTH	EA	ST	SO	UTH	
	Long F	ueville Rd	Acc	ess	Longi F	ueville Rd			Long F	ueville Rd	Ac	cess	Longi R	ueville Rd			Longi F	ueville Rd	Acc	ess	Longu R	leville d	
Peak Per	T	L	<u>R</u>	L	R	T	тот	Peak Per	T	L	<u>R</u>	L	<u>R</u>	T	тот	Peak Per	<u>T</u>	L	<u>R</u>	L	R	<u>T</u>	тот
0630 - 0730	184	0	1	0	0	279	464	0630 - 0730	15	0	0	0	0	8	23	0630 - 0730	199	0	1	0	0	287	487
0645 - 0745	188	0	1	0	1	323	513	0645 - 0745	16	0	0	0	0	12	28	0645 - 0745	204	0	1	0	1	335	541
0700 - 0800	190	0	1	0	1	342	534	0700 - 0800	13	0	0	0	0	13	26	0700 - 0800	203	0	1	0	1	355	560
0715 - 0815	179	0	2	1	1	363	546	0715 - 0815	9	0	0	0	0	14	23	0715 - 0815	188	0	2	1	1	377	569
0730 - 0830	207	0	3	2	1	384	597	0730 - 0830	6	0	0	0	0	16	22	0730 - 0830	213	0	3	2	1	400	619
0745 - 0845	218	0	4	2	0	441	665	0745 - 0845	5	0	0	0	0	13	18	0745 - 0845	223	0	4	2	0	454	683
0800 - 0900	265	0	3	3	0	446	717	0800 - 0900	5	0	0	0	0	14	19	0800 - 0900	270	0	3	3	0	460	736
0815 - 0915	260	0	2	2	0	442	706	0815 - 0915	7	0	0	0	0	12	19	0815 - 0915	267	0	2	2	0	454	725
0830 - 0930	235	0	1	1	1	385	623	0830 - 0930	8	0	0	0	0	9	17	0830 - 0930	243	0	1	1	1	394	640
PEAK HR	265	0	3	3	0	446	717	PEAK HR	5	0	0	0	0	14	19	PEAK HR	270	0	3	3	0	460	736

	R.O	.A.R.	DA	ГΑ											Cli	ent	: Varga Tra	ffic Plan	ning	
	Reliat	ble, Or	iginal &	& Authe	entic Res	ults									Job No	o/Name	: 6772 LAN	E COVE	Longuevi	lle Rd
D N	Ph.881	96847,	Mob 04	1823901	9										Day	/Date	: Tuesday 1	st May	2018	
										1		2		3						
										4		5		6						
			AM F	PEAK												TO	TAL VOLUMES			
			0800 -	0900						7		8		9		F	OR COUNT			
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	463			265									1086							
	449	5	0	270										626						
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		270	0	*										655						
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		460	0										1050							
	460	446	0	5										629						
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	L	.ongue	ville R	d								Lo	ongue	ville Rd						

	R.O	.A.R	. DA	ΔTA																			
	Relia	ble, O	riginal	& Au	thentic	: Resı	ılts	PEDS	NO	RTH	EA	AST	SO	JTH		PEDS	NO	RTH	EA	ST	SO	UTH	
	Ph.88	196847	, Mob C	41823	9019			Time Per	Longı R	ueville Rd	Acc	cess	Longı R	leville d	тот	Peak Per	Longi F	leville Rd	Acc	ess	Longı R	leville d	тот
								1530 - 1545	(0	1	1	()	11	1530 - 1630		1	2	22	(0	23
								1545 - 1600	(0		3	()	3	1545 - 1645		1	1	5	(5	16
Client	t	: Varg	ja Traf	fic Pla	nning			1600 - 1615		1		5	()	6	1600 - 1700		1	1	4	(0	15
Job No/N	ame	: 6772	2 LANI	E COV	E Lon	guevill	e Rd	1615 - 1630		0		3	()	3	1615 - 1715		0	1	4	(2	14
Day/Da	ate	: Tues	sday 1	st May	/ 2018			1630 - 1645		0		4	()	4	1630 - 1730		0	1	4	(2	14
								1645 - 1700		0		2	()	2	1645 - 1745		0	1	5	(<u>ა</u>	15
								1700 - 1715	(0	:	5	()	5	1700 - 1800		0	2	24	()	24
								1715 - 1730		0	:	3	()	3	1715 - 1815		0	2	23	()	23
								1730 - 1745	(0		5	()	5	1730 - 1830		0	2	29	()	29
								1745 - 1800	(0	1	1	()	11			_		-			45
								1800 - 1815		0		4	()	4	PEAK HR		0	1	5)	15
								1815 - 1830		0 1	6	9 \$ 5)	9								
								Per Ella		1	C	55		,	00								
Lights	NO	RTH	EA	ST	SO	UTH		Heavies	NO	RTH	EA	ST	SO	JTH		Combined	NO	RTH	EA	ST	SO	UTH	
	Long	ueville	4.00		Longu	ıeville			Longu	ueville	1.00		Longu	ieville			Longu	ıeville	4.4		Longı	leville	
	F	2d	ACC	ess	R	d			R	2d	ACC	cess	R	d			F	?d	ACC	ess	R	d	
Time Per	T	L	<u>R</u>	L	<u>R</u>	T	тот	Time Per	T	L	<u>R</u>	L	R	T	тот	Time Per	T	L	R	L	R	T	тот
1530 - 1545	70	0	0	0	0	82	152	1530 - 1545	4	0	0	0	0	0	4	1530 - 1545	74	0	0	0	0	82	156
1545 - 1600	59	0	1	0	0	68	128	1545 - 1600	7	0	0	0	0	5	12	1545 - 1600	66	0	1	0	0	73	140
1600 - 1615	63	1	0	0	0	73	137	1600 - 1615	3	0	0	0	0	2	5	1600 - 1615	66	1	0	0	0	75	142
1615 - 1630	56	1	0	0	0	73	130	1615 - 1630	5	0	0	0	0	4	9	1615 - 1630	61	1	0	0	0	77	139
1630 - 1645	83	0	0	1	0	74	158	1630 - 1645	0	0	0	0	0	2	2	1630 - 1645	83	0	0	1	0	76	160
1645 - 1700	80	0	0	0	0	99	179	1645 - 1700	2	0	0	0	0	2	4	1645 - 1700	82	0	0	0	0	101	183
1700 - 1715	86	0	0	0	0	96	182	1700 - 1715	1	0	0	0	0	1	2	1700 - 1715	87	0	0	0	0	97	184
1715 - 1730	59	0	1	1	1	85	147	1715 - 1730	2	0	0	0	0	3	5	1715 - 1730	61	0	1	1	1	88	152
1730 - 1745	95	3	1	0	1	82	182	1730 - 1745	1	0	0	0	0	1	2	1730 - 1745	96	3	1	0	1	83	184
1745 - 1800	63 72	1	0	0	0	82	140	1745 - 1800	4	0	0	0	0	2	0	1745 - 1800	57	1	0	0	0	84	152
1815 - 1830	67	1	1	0	0	76	1/5	1815 - 1830	3	0	0	0	0	1	2	1815 - 1830	74	3	1	0	0	77	1/0
Per End	854	10	4	2	2	970	1842	Per End	33	0	0	0	0	24	57	Per End	887	10	4	2	2	994	1899
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Lights	NO	RTH	EA	ST	SO	UTH		Heavies	NO	RTH	EA	AST	SO	JTH		<u>Combined</u>	NO	RTH	E4	ST	SO	JTH	
	Longi	leville d	Acc	ess	Longu	leville d			Longu	ueville Pd	Acc	cess	Longu	leville d			Longu	leville Dd	Acc	ess	Longu	ieville vd	
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1530 - 1630	249	<u> </u>	1		<u> </u>	206	547	1520 - 1620	10		<u> </u>		<u> </u>	<u> </u>	20	1530 - 1630	267	<u> </u>	1		<u> </u>	207	577
1545 - 1645	240	2	1	1	0	290	552	1545 1645	15	0	0	0	0	12	29	1545 - 1645	207	2	1	1	0	307	591
1600 - 1700	282	2	0	1	0	319	604	1600 - 1700	10	0	0	0	0	10	20	1600 - 1700	292	2	0	1	0	329	624
1615 - 1715	305	1	0	1	0	342	649	1615 - 1715	8	0	0	0	0	9	17	1615 - 1715	313	1	0	1	0	351	666
1630 - 1730	308	0	1	2	1	354	666	1630 - 1730	5	0	0	0	0	8	13	1630 - 1730	313	0	1	2	1	362	679
1645 - 1745	320	3	2	1	2	362	690	1645 - 1745	6	0	0	0	0	7	13	1645 - 1745	326	3	2	1	2	369	703
1700 - 1800	303	4	2	1	2	345	657	1700 - 1800	8	0	0	0	0	7	15	1700 - 1800	311	4	2	1	2	352	672
1715 - 1815	290	7	2	1	2	329	631	1715 - 1815	8	0	0	0	0	7	15	1715 - 1815	298	7	2	1	2	336	646
1730 - 1830	298	8	2	0	1	320	629	1730 - 1830	9	0	0	0	0	5	14	1730 - 1830	307	8	2	0	1	325	643
PEAK HR	320	3	2	1	2	362	690	PEAK HR	6	0	0	0	0	7	13	PEAK HR	326	3	2	1	2	369	703

	R.O	.A.R	. DATA									Clie	nt	: Varga Tra	ffic Pla	nning	
	Relia	ble, O	riginal & Aut	hentic Results								Job No/	Name	: 6772 LAN	E COV	E Long	ueville Rd
D P	Ph.88	196847	, Mob 0418239	019								Day/D	Date	: Tuesday	st May	/ 2018	
							1		2		3						
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			PM PEAK										TOTA	L VOLUMES			
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	L	ongue	ville Rd						L	ongue	eville Rd						

	R.O	.A.R.	DA	TA																			
	Relia	ble, Or	iginal	& Auth	hentic	Result	ts	PEDS	W	EST	so	UTH	EA	AST		PEDS	WE	ST	SO	UTH	EA	ST	1
	Ph.881	196847,	Mob 04	4182390	019			Time Per	Long	ueville	Kenn	neth St	Norwo	ood Rd	тот	Peak Per	Longu	ıeville	Kenn	eth St	Norwo	od Rd	тот
								0630 - 0645		0		0		0	0	0630 - 0730		1		6	(3	13
								0645 - 0700		0		1		1	2	0645 - 0745	:	2	1	1	8	3	21
Clien	t	: Varg	a Traff	ic Plan	ning			0700 - 0715		1		2		0	3	0700 - 0800	:	2	1	4	1	1	27
Job No/N	lame	: 6772	2 LANE	COVE	E Long	ueville	Rd	0715 - 0730		0		3		5	8	0715 - 0815		1	1	15	1	7	33
Day/Da	ate	: Tues	sday 1	st May	2018			0730 - 0745		1		5		2	8	0730 - 0830		1	1	4	1	2	27
								0745 - 0800		0		4		4	8	0745 - 0845	(C	1	0	1	5	25
								0800 - 0815		0		3		6	9	0800 - 0900	(2		6	1	3	19
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								0900 - 0915		0		0		2	2	PEAK HR		1	1	4	1	2	27
								0915 - 0930		0		2		3	5								
								Per End		2	2	23	3	50	55								
Lights	w	EST	so	ОТН	EA	ST		Heavies	w	EST	so	ОЛТН	EA	ST		Combined	WE	ST	SO	UTH	EA	ST	
	Long	ueville	Komm	C4	No mus			<u></u>	Long	ueville	Kana		Nomu				Longu	<i>ieville</i>	Kann	- 4h C4	News		1
	F	Rd	Kenn	eth St	Norwo				F	Rd	Kenn	ieth St	Norwo	boa ka			R	d	Kenn	emst	NOrwo		
Time Per	I	<u>R</u>	L	<u>R</u>	L	I	тот	Time Per	I	<u>R</u>	L	<u>R</u>	L	<u>T</u>	тот	Time Per	I	<u>R</u>	L	<u>R</u>	L	<u>T</u>	тот
0630 - 0645	202	26	16	19	5	76	344	0630 - 0645	1	0	0	0	0	0	1	0630 - 0645	203	26	16	19	5	76	345
0645 - 0700	276	32	32	16	7	131	494	0645 - 0700	2	0	0	1	0	0	3	0645 - 0700	278	32	32	17	7	131	497
0700 - 0715	318	35	36	33	6	125	553	0700 - 0715	2	0	3	1	1	0	7	0700 - 0715	320	35	39	34	7	125	560
0715 - 0730	276	25	33	41	4	129	508	0715 - 0730	5	1	0	0	0	1	7	0715 - 0730	281	26	33	41	4	130	515
0730 - 0745	381	33	38	35	3	208	698	0730 - 0745	6	0	0	3	0	1	10	0730 - 0745	387	33	38	38	3	209	708
0745 - 0800	344	24	39	51	4	174	636	0745 - 0800	2	0	0	2	0	5	9	0745 - 0800	346	24	39	53	4	179	645
0800 - 0815	398	37	35	32	4	221	727	0800 - 0815	0	0	1	1	0	1	3	0800 - 0815	398	37	36	33	4	222	730
0815 - 0830	368	35	48	35	6	205	697	0815 - 0830	1	1	0	0	0	0	2	0815 - 0830	369	36	48	35	6	205	699
0830 - 0845	335	38	41	18	4	166	602	0830 - 0845	0	0	1	1	1	2	5	0830 - 0845	335	38	42	19	5	168	607
0845 - 0900	370	50	38	17	8	163	646	0845 - 0900	1	0	0	0	0	3	4	0845 - 0900	371	50	38	17	8	166	650
0900 - 0915	277	33	32	16	2	150	510	0900 - 0915	1	1	1	1	0	0	4	0900 - 0915	278	34	33	17	2	150	514
0915 - 0930	273	37	29	20	8	124	491	0915 - 0930	1	0	1	0	0	1	3	0915 - 0930	274	37	30	20	8	125	494
PerEna	3818	405	417	333	61	1872	6906	PerEnd	22	3		10	2	14	58	PerEna	3840	408	424	343	63	1886	6964
Lights	W	EST	SO	UTH	EA	ST		Heavies	w	EST	SO	UTH	EA	AST		Combined	WE	ST	SO	UTH	EA	ST	
	Long	ueville	Kenn	eth St	Norwo	od Rd			Long	ueville	Kenr	neth St	Norwo	ood Rd			Longu	ıeville	Kenn	eth St	Norwo	od Rd	
	F	Rd							F	Rd							R	2d					
Peak Per	I	<u>R</u>	L	<u>R</u>	L	I	TOT	Peak Per	I	<u>R</u>	L	<u>R</u>	Ŀ	<u> </u>	TOT	Peak Per	I	<u>R</u>	L	<u>R</u>	L	<u> </u>	TOT
0630 - 0730	1072	118	117	109	22	461	1899	0630 - 0730	10	1	3	2	1	1	18	0630 - 0730	1082	119	120	111	23	462	1917
0645 - 0745	1251	125	139	125	20	593	2253	0645 - 0745	15	1	3	5	1	2	27	0645 - 0745	1266	126	142	130	21	595	2280
0700 - 0800	1319	117	146	160	17	636	2395	0700 - 0800	15	1	3	6	1	7	33	0700 - 0800	1334	118	149	166	18	643	2428
0715 - 0815	1399	119	145	159	15	732	2569	0715 - 0815	13	1	1	6	0	8	29	0715 - 0815	1412	120	146	165	15	740	2598
0730 - 0830	1491	129	160	153	17	808	2758	0730 - 0830	9	1	1	6	0	7	24	0730 - 0830	1500	130	161	159	17	815	2782
0745 - 0845	1445	134	163	136	18	766	2662	0745 - 0845	3	1	2	4	1	8	19	0745 - 0845	1448	135	165	140	19	774	2681
0800 - 0900	1471	160	162	102	22	755	2672	0800 - 0900	2	1	2	2	1	6	14	0800 - 0900	1473	161	164	104	23	761	2686
0815 - 0915	1350	156	159	86	20	684	2455	0815 - 0915	3	2	2	2	1	5	15	0815 - 0915	1353	158	161	88	21	689	2470
0830 - 0930	1255	158	140	71	22	603	2249	0830 - 0930	3	1	3	2	1	6	16	0830 - 0930	1258	159	143	73	23	609	2265
PEAK HR	1491	129	160	153	17	808	2758	PEAK HR	9	1	1	6	0	7	24	PEAK HR	1500	130	161	159	17	815	2782

A BAR	R.O.	A.R.	DAT	Ά											Clie	nt	: Varç	ga Traff	ic Plan	ining		
	Reliat	ole, Ori	iginal &	Authe	entic F	Results									Job No/	Name	: 677	2 LANE	COVE	E Long	ueville	Rd
D A	Ph.881	96847,	Mob 041	823901	9										Day/I	Date	: Tue	sday 1s	st May	2018		
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	R.O	.A.R	. DA	ΔTA																			
	Reliable, Original & Authe		thenti	c Resi	ılts	PEDS	WE	EST	SO	UTH	EA	ST		PEDS	WE	EST	SO	UTH	EA	ST			
DA	Ph.88	196847	, Mob 0	41823	9019			Time Per	Longi R	ueville Rd	Kenn	eth St	Norwo	od Rd	тот	Peak Per	Longi F	ueville Rd	Kenn	eth St	Norwo	od Rd	тот
								1530 - 1545		0		0	4	4	4	1530 - 1630		0	1	5	3	5	50
								1545 - 1600		0		2	1	3	15	1545 - 1645		0	2	4	3	9	63
Client		: Varg	a Traf	fic Pla	inning			1600 - 1615	(0		5	1	4	19	1600 - 1700	(0	2	2	3	3	55
Job No/N	ame	: 6772	2 LANE	E COV	/E Lon	guevill	e Rd	1615 - 1630	(0		8	4	4	12	1615 - 1715	(0	1	7	2	3	40
Day/Da	te	: Tues	sday 1	st Ma	y 2018	3		1630 - 1645	(0		9	8	3	17	1630 - 1730	(0	1	1	3	1	42
								1645 - 1700		0		0		7	7	1645 - 1745	(0	2	2	2	8	30
								1700 - 1715		0		0	4	4	4	1700 - 1800	(0	3	3	2	9	32
								1715 - 1730		0		2	1	2	14	1715 - 1815		0	4	4	3	2	36
								1730 - 1745		0		0	!	5	5	1730 - 1830		0	2	2	2	4	26
								1745 - 1800		0		1	8	3	9								
								1800 - 1815		0		1		7	8	PEAK HR	(0		2	2	8	30
								1815 - 1830		0		0	4	4	4								
								Per End	(0	2	28	9	0	118								
	WEST								-								-				-		-
Lights	WEST Longuevill		SO	UTH	EA	AST		Heavies	WE	EST	SO	UTH	EA	ST		Combined	WE	EST	SO	UTH	EA	ST	
	Longi F	ieville d	R L		Norwo	ood Rd			Longi R	d Rd	Kenn	eth St	Norwo	od Rd			Longi F	ueville Rd	Kenne	eth St	Norwo	od Rd	
Time Per	<u>T</u> 5 153 3		L	<u>R</u>	L	T	тот	Time Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	T	тот	Time Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	<u>T</u>	тот
1530 - 1545	153	38	46	17	9	255	518	1530 - 1545	1	0	1	0	0	1	3	1530 - 1545	154	38	47	17	9	256	521
1545 - 1600	138	38	25	13	6	316	536	1545 - 1600	1	0	2	1	1	2	7	1545 - 1600	139	38	27	14	7	318	543
1600 - 1615	116	41	33	10	5	289	494	1600 - 1615	5	1	2	1	0	2	11	1600 - 1615	121	42	35	11	5	291	505
1615 - 1630	149	32	43	11	4	307	546	1615 - 1630	2	2	3	0	0	1	8	1615 - 1630	151	34	46	11	4	308	554
1630 - 1645	154	52	34	14	11	303	568	1630 - 1645	2	0	0	1	0	1	4	1630 - 1645	156	52	34	15	11	304	572
1645 - 1700	181	51	41	18	4	357	652	1645 - 1700	1	0	2	0	0	2	5	1645 - 1700	182	51	43	18	4	359	657
1700 - 1715	156	49	42	16	5	283	551	1700 - 1715	0	0	1	1	0	0	2	1700 - 1715	156	49	43	17	5	283	553
1730 - 1745	195	49	39 42	21	5	303	647	1730 - 1745	2	1	0	1	0	0	3	1713 - 1730	197	49	40	22	5	3/1	650
1745 - 1800	179	44	39	21	7	347	637	1745 - 1800	1	0	1	0	0	0	2	1745 - 1800	180	40	40	21	7	347	639
1800 - 1815	172	43	40	13	7	271	546	1800 - 1815	2	0	0	1	0	0	3	1800 - 1815	174	43	40	14	7	271	549
1815 - 1830	185	47	35	13	10	264	554	1815 - 1830	1	0	1	1	0	0	3	1815 - 1830	186	47	36	14	10	264	557
Per End	1972	528	459	181	80	3696	6916	Per End	19	4	14	7	1	9	54	Per End	1991	532	473	188	81	3705	6970
Lights	w	ST	so	UTH	EA	AST		Heavies	w	EST	so	UTH	EA	ST		Combined	W	EST	SO	UTH	EA	ST	Ì
	Longi F	west South EAST Longueville Rd Kenneth St Norwood Rd		ood Rd			Longi F	ueville Rd	Kenn	eth St	Norwo	od Rd			Longi F	ueville Rd	Kenn	eth St	Norwo	od Rd			
Peak Per	т	R	L	R	L	т	тот	Peak Per	т	R	L	R	L	т	тот	Peak Per	т	R	L	R		т	тот
1530 - 1630	556	149	147	51	24	1167	2094	1530 - 1630	9	3	8	2	1	6	29	1530 - 1630	565	152	155	53	25	1173	2123
1545 - 1645	557	163	135	48	26	1215	2144	1545 - 1645	10	3	7	3	1	6	30	1545 - 1645	567	166	142	51	27	1221	2174
1600 - 1700	600	176	151	53	24	1256	2260	1600 - 1700	10	3	7	2	0	6	28	1600 - 1700	610	17.9	158	55	24	1262	2288
1615 - 1715	640	184	160	59	24	1250	2317	1615 - 1715	5	2	6	2	0	4	19	1615 - 1715	645	186	166	61	24	1254	2336
1630 - 1730	686	201	156	62	27	1306	2438	1630 - 1730	5	0	4	2	0	3	14	1630 - 1730	691	201	160	64	27	1309	2452
1645 - 1745	726	193	164	69	21	1344	2517	1645 - 1745	4	1	4	2	0	2	13	1645 - 1745	730	194	168	71	21	1346	2530
1700 - 1800	724	186	162	72	24	1334	2502	1700 - 1800	4	1	3	2	0	0	10	1700 - 1800	728	187	165	74	24	1334	2512
1715 - 1815	7/0	180	160	60	24	1322	2407	1715 - 1815	6	1	2	2	0	0	11	1715 - 1815	7/6	181	162	71	26	1322	2502
1730 - 1830	730	178	156	68	20	1222	2384	1730 - 1830	5	1	2	<u>८</u> २	0	0	11	1730 - 1830	735	179	158	71	20	1223	2305
ΡΕΔΚ ΗΡ	726	193	164	69	23	1344	2517		4	1	<u> </u>	2	0	2	13		730	194	168	71	21	1346	2530
Peak Per 1530 - 1630 1545 - 1645 1600 - 1700 1615 - 1715 1630 - 1730 1645 - 1745 1700 - 1800 1715 - 1815 1730 - 1830 PEAK HR	<u>T</u> 5556 557 600 640 686 726 724 740 730 726	R 149 163 176 184 201 193 186 180 178 193	L 147 135 151 160 156 164 162 160 156 164	R 51 48 53 59 62 69 72 69 68 69	L 24 26 24 24 27 21 24 26 29 21	I 1167 1215 1256 1250 1306 1344 1334 1322 1223 1344	TOT 2094 2144 2260 2317 2438 2517 2502 2497 2384 2517	Peak Per 1530 - 1630 1545 - 1645 1600 - 1700 1615 - 1715 1630 - 1730 1645 - 1745 1700 - 1800 1715 - 1815 1730 - 1830 PEAK HR	I 9 10 5 4 6 5 4 6 5	R 3 3 2 0 1 1 1 1 1	L 8 7 6 4 4 3 2 2 2	R 2 3 2 2 2 2 2 2 2 2 2 2 3 3	L 1 0 0 0 0 0 0 0 0 0 0 0 0	T 6 6 4 3 2 0 0 0 2	TOT 29 30 28 19 14 13 10 11 11 13 13	Peak Per 1530 - 1630 1545 - 1645 1600 - 1700 1615 - 1715 1630 - 1730 1645 - 1745 1700 - 1800 1715 - 1815 1730 - 1830 PEAK HR	I 565 567 610 645 691 730 728 746 735 730	R 152 166 179 186 201 194 187 181 179 181 179 194	L 155 142 158 166 160 168 165 162 158 168	R 53 51 55 61 64 71 74 71 71 71	L 25 27 24 24 27 21 24 26 29 29 21	<u>T</u> 1173 1221 1262 1254 1309 1346 1334 1322 1223 1346	TO 212 217 228 233 245 253 251 250 239 253

	R.O	A.R.	DA	ТА											C	Client		: Varga Tra	fic Pla	nning		
	Reliat	ole, Or	iginal	& Au	thentic	Results	;								Job I	No/Nam	ne	: 6772 LAN	E COV	E Lon	queville	e Rd
DA	Ph.881	, 96847,	Mob 04	18239	9019										Da	y/Date		: Tuesday ?	st May	/ 2018	,	
																			-			
1		2		3																		
																Т	ΌΤΑ	L VOLUMES				
4		5		6	<u>PM F</u>	PEAK					N						FC	OR COUNT				
					1645 ·	- 1745					A							PERIOD				
7		8		9								>										
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Lor	auovil	o Pd						No	rwooo	I Dd			23 1 000			.525 —			20	Z100	Dd	
<u>L01</u>	5 919	924						6	795	801 -	-		Long	ueviik	, Nu			n		woou	Nu	
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	R.O.	.A.R.	DA	ТА																			
i d	Reliat	ole, Or	iginal	& Auth	nentic l	Result	s	PEDS	WE	ST	NO	RTH	EA	ST		PEDS	WE	EST	NO	RTH	EA	ST	
D	Ph.881	, 96847,	Mob 04	182390)19			Time Per	Rive	r Rd	Long	ueville Da	Longu	leville	тот	Peak Per	Rive	er Rd	Long	ueville 2d	Longu	leville	тот
				0630 - 0645	:	3		0	<u> </u>	0	3	0630 - 0730	1	4		3)	17				
								0645 - 0700	:	3		2	(0	5	0645 - 0745	2	21	:	3	()	24
Clien	t	: Varg	a Traff	ic Plan	ning			0700 - 0715		3		0	(0	3	0700 - 0800	2	21	:	5	()	26
Job No/N	lame	: 6772	2 LANE	COVE	E Long	ueville	Rd	0715 - 0730		5		1	(0	6	0715 - 0815	2	20	;	8	()	28
Day/Da	ate	: Tues	day 1	st May	2018			0730 - 0745	1	0		0	(0	10	0730 - 0830	2	23		7	()	30
								0745 - 0800		3		4	(0	7	0745 - 0845	1	7	;	8	()	25
								0800 - 0815		2		3	(0	5	0800 - 0900	1	7		6	()	23
								0815 - 0830		В		0	(0	8	0815 - 0915	2	20	:	3	()	23
								0830 - 0845	4	4		1	(0	5	0830 - 0930	1	4	4	5	()	19
								0845 - 0900		3		2	(0	5								
								0900 - 0915		5		0	(0	5	PEAK HR	1	7	(6	()	23
								0915 - 0930		2		2	(0	4								
								Per End	5	1	1	5	(0	66								
Lights	WE	ST.	NO	ртн	EA	ST	1	Heavies	WE	-ет	NO	ртн	EA	sт	1	Combined	WE	-ет	NO	ртн	FA	sт	
Lights	VVL	-51	Long	veville	Long	ieville		rieavies		-51	Long	veville	Long	leville		combined	VVL		Long	veville	Longi	eville	
	Rive	r Rd	F	Rd	F	2d			Rive	er Rd	F	Rd	R	2d			Rive	er Rd	F	Rd	R	d	
Time Per	I	L	<u>R</u>	L	<u>R</u>	I	тот	Time Per	I	L	<u>R</u>	L	<u>R</u>	I	тот	Time Per	Ι	L	R	L	<u>R</u>	Τ	тот
0630 - 0645	240	10	0	43	29	87	409	0630 - 0645	0	2	0	1	0	0	3	0630 - 0645	240	12	0	44	29	87	412
0645 - 0700	271	15	0	36	61	90	473	0645 - 0700	0	1	1	3	0	0	5	0645 - 0700	271	16	1	39	61	90	478
0700 - 0715	285	19	0	61	41	116	522	0700 - 0715	0	2	4	1	1	3	11	0700 - 0715	285	21	4	62	42	119	533
0715 - 0730	303	34	0	38	66	132	573	0715 - 0730	4	2	2	3	0	1	12	0715 - 0730	307	36	2	41	66	133	585
0730 - 0745	346	21	0	46	62	157	632	0730 - 0745	3	5	0	2	1	2	13	0730 - 0745	349	26	0	48	63	159	645
0745 - 0800	345	27	0	38	66	151	627	0745 - 0800	1	2	0	1	0	3	7	0745 - 0800	346	29	0	39	66	154	634
0800 - 0815	327	34	0	51	52	165	629	0800 - 0815	0	3	1	0	1	1	6	0800 - 0815	327	37	1	51	53	166	635
0815 - 0830	317	33	0	68	82	174	674	0815 - 0830	1	3	1	1	0	1	7	0815 - 0830	318	36	1	69	82	175	681
0830 - 0845	335	60	0	55	87	140	677	0830 - 0845	0	3	1	0	1	2	7	0830 - 0845	335	63	1	55	88	142	684
0845 - 0900	313	41	0	88	64	127	633	0845 - 0900	0	2	0	1	1	1	5	0845 - 0900	313	43	0	89	65	128	638
0900 - 0915	261	31	0	60	51	142	545	0900 - 0915	1	1	2	1	1	2	8	0900 - 0915	262	32	2	61	52	144	553
0915 - 0930	244	21	0	45	45	112	467	0915 - 0930	0	1	2	1	0	40	4	0915 - 0930	244	22	2	46	45	112	4/1
PerEna	3587	346	U	629	706	1593	6861	PerEna	10	21	14	15	6	16	88	PerEnd	3597	3/3	14	644	/12	1609	6949
Lights	WE	ST	NO	RTH	EA	ST		Heavies	WE	ST	NO	RTH	EA	ST		Combined	WE	EST	NO	RTH	EA	ST	
	Rive	r Rd	Long	ueville	Longu	leville			Rive	r Rd	Long	ueville	Longu	leville			Rive	er Rd	Long	ueville	Longu	leville	
Peak Per	т	1	P	1		<u>и</u> т	TOT	Peak Per	т	1	P	1		т	тот	Peak Per	т		P	1		<u>и</u> Т	тот
0630 - 0730	1099	78	0	178	197	425	1977	0630 - 0730	4	7	7	8	1	4	31	0630 - 0730	1103	<u> </u>	7	186	198	429	2008
0645 - 0745	1205	89	0	181	230	495	2200	0645 - 0745	7	10	7	9	2	6	41	0645 - 0745	1212	99	7	190	232	501	2000
0700 - 0800	1279	101	0	183	235	556	2354	0700 - 0800	8	11	6	7	2	9	43	0700 - 0800	1287	112	6	190	237	565	2397
0715 - 0815	1321	116	0	173	246	605	2461	0715 - 0815	8	12	3	6	2	7	38	0715 - 0815	1329	128	3	179	248	612	2499
0730 - 0830	1335	115	0	203	262	647	2562	0730 - 0830	5	13	2	4	2	7	33	0730 - 0830	1340	128	2	207	264	654	2595
0745 - 0845	1324	154	0	212	287	630	2607	0745 - 0845	2	11	3	2	2	7	27	0745 - 0845	1326	165	3	214	289	637	2634
0800 - 0900	1292	168	0	262	285	606	2613	0800 - 0900	1	11	3	2	3	5	25	0800 - 0900	1293	179	3	264	288	611	2638
0815 - 0915	1226	165	0	271	284	583	2529	0815 - 0915	2	9	4	3	3	6	27	0815 - 0915	1228	174	4	274	287	589	2556
0830 - 0930	1153	153	0	248	247	521	2322	0830 - 0930	1	7	5	3	3	5	24	0830 - 0930	1154	160	5	251	250	526	2346
PEAK HR	1292	168	0	262	285	606	2613	PEAK HR	1	11	3	2	3	5	25	PEAK HR	1293	179	3	264	288	611	2638

	R.O .	A.R.	DAT	Α												C	lient	: Va	rga Traf	fic Plan	ning		
	Reliat	le, Ori	iginal 8	Auth	entic l	Result	S									Job N	lo/Name	: 67	72 LANE	E COVE	E Longi	ueville R	d
DN	Ph.881	96847,	Mob 04	182390	19											Da	y/Date	: Tu	esday 1	st May	2018		
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														←	1623	1593	30		-	2321	2299	22	

	R.O	.A.R	. DA	ATA																			
	Reliable, Original & Au		l & Au	thentio	: Resı	ılts	PEDS	WE	EST	NO	RTH	EA	ST		PEDS	WE	EST	NO	RTH	EA	ST		
D N	Ph.88	196847	, Mob (0418239	9019			Time Per	Rive	er Rd	Long	ueville 2d	Longu	leville d	тот	Peak Per	Rive	er Rd	Long	ueville 2d	Longu R	eville d	тот
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								1545 - 1600		6		0	(C	6	1545 - 1645	1	4		1	C)	15
Client	t	: Varg	ga Tra	fic Pla	nning			1600 - 1615	1	2		1	(C	3	1600 - 1700	1	1		1	C)	12
Job No/N	ame	: 677	2 LAN	E COV	/E Lon	guevill	e Rd	1615 - 1630	:	3		0	(D	3	1615 - 1715	1	1		2	C)	13
Day/Da	ate	: Tue	sday 1	st May	y 2018			1630 - 1645	:	3		0	()	3	1630 - 1730	1	0		2	0)	12
								1645 - 1700		3		0	(0	3	1645 - 1745	1	0		3	0)	13
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Lights	WE	EST	NO	RTH	EA	ST		<u>Heavies</u>	WE	EST	NO	RTH	EA	ST		<u>Combined</u>	W	EST	NO	RTH	EA	ST	
	Rive	er Rd	Long	Rd	Longt F	leville Rd			Rive	er Rd	F	leville Rd	R	d			Rive	er Rd	F	Rd	R	d	
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1545 - 1600	121	18	1	56	53	275	524	1545 - 1600	2	3	5	2	2	3	17	1545 - 1600	123	21	6	58	55	278	541
1600 - 1615	103	16	0	60	59	287	525	1600 - 1615	4	0	3	0	2	1	10	1600 - 1615	107	16	3	60	61	288	535
1615 - 1630	112	16	0	56	56	294	534	1615 - 1630	2	1	3	2	3	1	12	1615 - 1630	114	17	3	58	59	295	546
1630 - 1645	125	13	0	84	67	273	562	1630 - 1645	1	2	0	0	0	2	5	1630 - 1645	126	15	0	84	67	275	567
1645 - 1700	139	23	0	79	76	342	659	1645 - 1700	0	0	1	1	2	1	5	1645 - 1700	139	23	1	80	78	343	664
1700 - 1715	154	22	1	85	74	258	594	1700 - 1715	1	0	1	0	1	0	3	1700 - 1715	155	22	2	85	75	258	597
1713 - 1730	140	15	0	04	70	328	676	1713 - 1730	1	2	0	1	0	0	3	1713 - 1730	149	16	0	03	70	328	679
1745 - 1800	149	23	0	64	64	294	594	1745 - 1800	2	1	4	0	1	0	8	1745 - 1800	151	24	4	64	65	294	602
1800 - 1815	168	15	0	76	65	257	581	1800 - 1815	1	1	0	1	0	0	3	1800 - 1815	169	16	0	77	65	257	584
1815 - 1830	133	14	0	65	60	215	487	1815 - 1830	0	0	2	1	1	0	4	1815 - 1830	133	14	2	66	61	215	491
Per End	1650	215	2	851	774	3419	6911	Per End	15	11	23	10	13	10	82	Per End	1665	226	25	861	787	3429	6993
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Lights			Lona	ueville	Lona	Jeville		<u>neuvics</u>			Lona	ueville	Lona	ieville		oombined			Long	ueville	Lonau	ieville	
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Peak Per	Τ	L	R	L	<u>R</u>	I	тот	Peak Per	I	L	<u>R</u>	L	<u>R</u>	<u>T</u>	тот	Peak Per	T	L	<u>R</u>	L	<u>R</u>	<u>T</u>	тот
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1545 - 1645	461	63	1	256	235	1129	2145	1545 - 1645	9	6	11	4	7	7	44	1545 - 1645	470	69	12	260	242	1136	2189
1600 - 1700	479	68	0	279	258	1196	2280	1600 - 1700	7	3	7	3	7	5	32	1600 - 1700	486	71	7	282	265	1201	2312
1615 - 1715	530	74	1	304	273	1167	2349	1615 - 1715	4	3	5	3	6	4	25	1615 - 1715	534	77	6	307	279	1171	2374
1630 - 1730	566	79	1	312	282	1211	2451	1630 - 1730	3	4	3	2	4	3	19	1630 - 1730	569	83	4	314	286	1214	2470
1645 - 1745	611	81	1	321	285	1266	2565	1645 - 1745	3	3	3	3	4	1	17	1645 - 1745	614	84	4	324	289	1267	2582
1700 - 1800	621	81	1	306	273	1218	2500	1700 - 1800	5	4	6	2	3	0	20	1700 - 1800	626	85	7	308	2/6	1218	2520
1730 - 1830	620	67	0	297	264	1/1/	248/	1715 - 1815	5 4	2	5	3	2	0	<u>∠</u> 0	1730 - 1835	624	79	5	300	200 261	1217	2307 2356
1730 - 1830	020	07	U	290	209	1094	2330	1730 - 1830	4	3	U	3	2	U	10	1730 - 1830	024	10	U	301	201	1094	2000
PEAK HR	611	81	1	321	285	1266	2565	PEAK HR	3	3	3	3	4	1	17	PEAK HR	614	84	4	324	289	1267	2582

A STATE	R.O.	A.R	. DA	ТА												Clien	t : \	√arga Trafl	ic Plar	nning			
	Reliat	ole, Ol	riginal	& Aut	hentic	: Resu	ılts									Job No/N	lame : 6	6772 LANE	COV	E Long	gueville	Rd	
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APPENDIX C

RMS IDM DATA

Report: Periodic statistics for site 936

15 minute intervals From 12:00:00 AM to 11:59:59 PM, on 15 August 2018

Period: 12:00:00 AM to 12:15:00 AM

Data	Freq.	Min	Max	Avg	Total
? phase	1	5	5	5	5
A phase	4	17	669	215	860
C phase	1	12	12	12	12
D phase	2	11	12	11	23

Period: 12:15:00 AM to 12:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	253	635	444	888
C phase	1	12	12	12	12

Period: 12:30:00 AM to 12:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 12:45:00 AM to 1:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	151	738	444	889
D phase	1	11	11	11	11

Period: 1:00:00 AM to 1:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 1:15:00 AM to 1:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 1:30:00 AM to 1:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 1:45:00 AM to 2:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:00:00 AM to 2:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:15:00 AM to 2:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:30:00 AM to 2:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:45:00 AM to 3:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:00:00 AM to 3:15:00 AM

Data	Freq.	Min	Мах	Avg	Total
A phase	1	900	900	900	900

Period: 3:15:00 AM to 3:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:30:00 AM to 3:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	107	781	444	888
C phase	1	12	12	12	12

Period: 3:45:00 AM to 4:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	197	691	444	888
C phase	1	12	12	12	12

Period: 4:00:00 AM to 4:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	239	649	444	888
D phase	1	12	12	12	12

Period: 4:15:00 AM to 4:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	95	660	292	876
C phase	2	12	12	12	24

Period: 4:30:00 AM to 4:45:00 AM

Dutu	. WIIN	l Max	AVg	lotal
A phase 1	900	900	900	900

Period: 4:45:00 AM to 5:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	73	507	292	876
C phase	2	12	12	12	24

Period: 5:00:00 AM to 5:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	4	20	699	216	864
C phase	3	12	12	12	36

Period: 5:15:00 AM to 5:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	16	211	102	816
C phase	6	12	12	12	72
D phase	1	12	12	12	12

Period: 5:30:00 AM to 5:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	6	1	268	140	840
C phase	4	12	12	12	48
D phase	1	12	12	12	12

Period: 5:45:00 AM to 6:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	16	206	100	804
C phase	7	7	18	12	85
D phase	1	11	11	11	11
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
Ped 2	1				

Period: 6:00:00 AM to 6:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	44	128	77	775
C phase	9	5	18	12	113
D phase	1	12	12	12	12
Nominal CL	3	28	60	49	
Active CL	3	28	60	49	
Ped 1	1				
Ped 2	2				

Period: 6:15:00 AM to 6:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	35	117	70	705
C phase	9	12	18	14	131
D phase	5	12	13	12	64
Ped 2	2				

Period: 6:30:00 AM to 6:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	33	102	49	686
C phase	12	3	19	13	162
D phase	4	12	15	13	52
Ped 2	2				

Period: 6:45:00 AM to 7:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	24	69	47	670
C phase	11	9	19	14	161
D phase	5	12	18	13	69
Nominal CL	6	69	82	75	
Active CL	6	69	82	75	
Ped 2	1				

Period: 7:00:00 AM to 7:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	12	4	83	52	627
C phase	10	13	22	16	169
D phase	7	13	24	14	104
Nominal CL	10	75	90	84	
Active CL	10	75	90	84	
Ped 2	2				

Period: 7:15:00 AM to 7:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	51	87	65	652
C phase	9	16	31	19	175
D phase	5	13	16	14	73
Nominal CL	8	90	100	95	
Active CL	8	90	100	95	
Ped 2	4				

Period: 7:30:00 AM to 7:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	26	91	62	624
C phase	9	18	28	20	187
D phase	6	12	16	14	89
Nominal CL	7	96	115	105	
Active CL	7	96	115	105	
Ped 1	1				
Ped 2	3				

Period: 7:45:00 AM to 8:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	42	88	65	654
C phase	9	18	27	21	191
D phase	4	13	15	13	55
Nominal CL	9	92	105	98	
Active CL	9	92	105	98	
Ped 1	1				
Ped 2	5				

Period: 8:00:00 AM to 8:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	36	90	71	647
C phase	9	13	33	20	181
D phase	5	12	16	14	72
Nominal CL	8	100	112	106	
Active CL	8	100	112	106	
Ped 1	1				
Ped 2	7				

Period: 8:15:00 AM to 8:30:00 AM

Data	Freq.	Min	Max	Avg	Total			
A phase	8	60	100	79	637			
C phase	9	6	19	15	139			
D phase	8	12	23	15	124			
Nominal CL	8	105	120	112				
Active CL	8	105	120	112				

Ped 1	2		
Ped 2	3		

Period: 8:30:00 AM to 8:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	43	96	80	642
C phase	8	5	19	17	138
D phase	8	12	16	15	120
Nominal CL	4	117	120	118	
Active CL	4	117	120	118	
Ped 2	4				

Period: 8:45:00 AM to 9:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	38	94	71	640
C phase	8	2	19	16	135
D phase	8	14	16	15	125
Nominal CL	7	93	120	107	
Active CL	7	93	120	107	
Ped 1	1				
Ped 2	5				

Period: 9:00:00 AM to 9:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	36	142	64	648
C phase	9	12	23	17	154
D phase	7	12	16	14	98
Nominal CL	11	62	97	81	
Active CL	11	62	97	81	
Ped 2	3				

Period: 9:15:00 AM to 9:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	13	14	66	46	606
C phase	12	12	23	17	205
D phase	7	11	13	12	89
Nominal CL	12	64	79	73	
Active CL	12	64	79	73	
Ped 1	2				
Ped 2	4				

Period: 9:30:00 AM to 9:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	34	79	45	639
C phase	11	5	24	15	168
D phase	7	12	14	13	93
Nominal CL	6	60	83	73	
Active CL	6	60	83	73	
Ped 1	1				
Ped 2	3				

Period: 9:45:00 AM to 10:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	11	95	41	625

C phase	13	12	19	14	185
D phase	7	12	13	12	90
Ped 1	2				
Ped 2	1				

Period: 10:00:00 AM to 10:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	21	53	38	612
C phase	9	12	20	17	155
D phase	9	11	20	14	133
Ped 2	4				

Period: 10:15:00 AM to 10:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	1	58	41	615
C phase	10	12	20	15	156
D phase	10	11	14	12	129
Nominal CL	7	64	76	71	
Active CL	7	64	76	71	
Ped 2	2				

Period: 10:30:00 AM to 10:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	15	95	44	623
C phase	11	12	19	14	159
D phase	9	12	15	13	118
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
Ped 2	3				

Period: 10:45:00 AM to 11:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	28	59	42	638
C phase	12	8	20	15	189
D phase	5	12	18	14	73
Nominal CL	4	60	67	64	
Active CL	4	60	67	64	
Ped 1	2				
Ped 2	3				

Period: 11:00:00 AM to 11:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	29	96	41	583
C phase	12	4	20	14	171
D phase	11	11	19	13	146
Ped 1	2				
Ped 2	3				

Period: 11:15:00 AM to 11:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	28	54	41	629
C phase	10	8	20	14	146
D phase	9	12	17	13	125
Nominal CL	1	69	69	69	

Active CL	1	69	69	69	
Ped 2	2				

Period: 11:30:00 AM to 11:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	10	57	40	610
C phase	13	2	20	13	173
D phase	9	11	17	13	117
Nominal CL	7	60	67	63	
Active CL	7	60	67	63	

Period: 11:45:00 AM to 12:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	18	46	37	602
C phase	13	12	20	14	194
D phase	8	12	15	13	104
Ped 1	1				
Ped 2	2				

Period: 12:00:00 PM to 12:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	15	105	41	623
C phase	10	12	20	16	165
D phase	8	12	20	14	112
Ped 2	2				

Period: 12:15:00 PM to 12:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	20	104	43	613
C phase	13	12	21	16	210
D phase	6	12	13	12	77
Nominal CL	7	60	75	67	
Active CL	7	60	75	67	
Ped 2	1				

Period: 12:30:00 PM to 12:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	13	51	38	613
C phase	13	12	20	13	181
D phase	8	12	16	13	106
Ped 1	1				

Period: 12:45:00 PM to 1:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	18	101	43	645
C phase	10	12	20	14	144
D phase	9	11	14	12	111
Ped 1	1				
Ped 2	1				

Period: 1:00:00 PM to 1:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	21	114	43	649

C phase	8	12	20	13	109
D phase	11	11	16	12	142

Period: 1:15:00 PM to 1:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	22	113	42	631
C phase	12	12	18	13	163
D phase	8	11	18	13	106
Ped 2	1				

Period: 1:30:00 PM to 1:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	22	97	42	636
C phase	12	12	19	13	159
D phase	8	12	15	13	105
Ped 1	1				
Ped 2	1				

Period: 1:45:00 PM to 2:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	21	48	37	604
C phase	14	12	20	13	195
D phase	8	12	13	12	101
Ped 2	1				

Period: 2:00:00 PM to 2:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	14	101	40	613
C phase	13	12	20	14	182
D phase	8	12	16	13	105
Ped 1	1				

Period: 2:15:00 PM to 2:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	21	64	43	609
C phase	12	12	18	14	172
D phase	10	3	14	11	119
Nominal CL	7	60	80	71	
Active CL	7	60	80	71	
Ped 1	2				
Ped 2	3				

Period: 2:30:00 PM to 2:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	34	59	43	614
C phase	12	3	20	14	170
D phase	9	10	14	12	116
Nominal CL	5	67	82	74	
Active CL	5	67	82	74	
Ped 2	1				

Period: 2:45:00 PM to 3:00:00 PM

Data	Freq.	Min	Max	Avg	Total

A phase	14	29	65	39	550
C phase	14	8	21	13	188
D phase	12	12	20	13	162
Nominal CL	6	60	74	67	
Active CL	6	60	74	67	
Ped 2	2				

Period: 3:00:00 PM to 3:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	29	65	42	558
C phase	13	5	20	13	179
D phase	12	12	16	13	163
Nominal CL	7	60	94	77	
Active CL	7	60	94	77	
Ped 1	1				
Ped 2	3				

Period: 3:15:00 PM to 3:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	39	70	56	619
C phase	10	4	19	14	142
D phase	10	13	16	13	139
Nominal CL	8	74	92	82	
Active CL	8	74	92	82	
Ped 2	3				

Period: 3:30:00 PM to 3:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	19	75	52	633
C phase	10	4	18	13	138
D phase	9	13	19	14	129
Nominal CL	11	62	99	77	
Active CL	11	62	99	77	
Ped 2	2				

Period: 3:45:00 PM to 4:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	3	89	43	559
C phase	11	12	19	17	191
D phase	10	13	17	15	150
Nominal CL	5	60	105	93	
Active CL	5	60	105	93	
Ped 1	2				
Ped 2	7				

Period: 4:00:00 PM to 4:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	52	81	64	641
C phase	8	17	19	18	144
D phase	8	12	17	14	115
Nominal CL	9	90	100	94	
Active CL	9	90	100	94	
Ped 1	2				
Ped 2	4				

Period: 4:15:00 PM to 4:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	4	85	55	606
C phase	10	15	20	17	175
D phase	8	13	18	14	119
Nominal CL	10	86	103	94	
Active CL	10	86	103	94	
Ped 2	4				

Period: 4:30:00 PM to 4:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	16	64	49	599
C phase	11	12	21	15	175
D phase	9	13	15	14	126
Nominal CL	9	71	96	82	
Active CL	9	71	96	82	
Ped 1	2				
Ped 2	2				

Period: 4:45:00 PM to 5:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	34	75	50	608
C phase	11	12	19	14	162
D phase	10	12	14	13	130
Nominal CL	12	63	86	75	
Active CL	12	63	86	75	
Ped 1	1				
Ped 2	3				

Period: 5:00:00 PM to 5:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	58	88	69	621
C phase	8	15	19	17	139
D phase	10	1	20	14	140
Nominal CL	7	92	111	101	
Active CL	7	92	111	101	
Ped 1	3				
Ped 2	3				

Period: 5:15:00 PM to 5:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	20	97	61	611
C phase	9	15	19	17	156
D phase	9	5	19	14	133
Nominal CL	9	89	106	97	
Active CL	9	89	106	97	
Ped 1	1				
Ped 2	4				

Period: 5:30:00 PM to 5:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	8	47	93	80	645
C phase	8	17	22	19	152
D phase	7	6	17	14	103
Nominal CL	5	112	120	116	
Active CL	5	112	120	116	
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Ped 1	4				
Ped 2	4				

Period: 5:45:00 PM to 6:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	8	56	88	76	613
C phase	8	17	19	18	147
D phase	9	10	18	15	140
Nominal CL	8	98	120	111	
Active CL	8	98	120	111	
Ped 1	2				
Ped 2	5				

Period: 6:00:00 PM to 6:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	14	64	53	639
C phase	9	12	19	14	133
D phase	10	1	16	12	128
Nominal CL	10	60	97	78	
Active CL	10	60	97	78	
Ped 2	2				

Period: 6:15:00 PM to 6:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	15	62	42	558
C phase	12	12	19	15	186
D phase	12	12	14	13	156
Nominal CL	9	60	88	76	
Active CL	9	60	88	76	
Ped 1	2				
Ped 2	4				

Period: 6:30:00 PM to 6:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	1	55	35	575
C phase	12	12	20	14	170
D phase	11	12	21	14	155
Nominal CL	3	70	77	74	
Active CL	3	70	77	74	
Ped 1	1				
Ped 2	2				

Period: 6:45:00 PM to 7:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	22	58	42	588
C phase	11	12	19	15	168
D phase	11	12	15	13	144
Nominal CL	9	60	78	68	
Active CL	9	60	78	68	
Ped 2	4				

Period: 7:00:00 PM to 7:15:00 PM

Data	Freq.	Min	Max	Avg	Total

A phase	14	3	97	45	640
C phase	9	12	18	14	134
D phase	9	13	17	14	126
Nominal CL	8	60	75	68	
Active CL	8	60	75	68	
Ped 2	3				

Period: 7:15:00 PM to 7:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	15	105	44	671
C phase	10	12	17	13	132
D phase	8	11	13	12	97
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	

Period: 7:30:00 PM to 7:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	26	94	44	661
C phase	8	12	18	13	111
D phase	10	11	15	12	128
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
Ped 2	1				

Period: 7:45:00 PM to 8:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	31	101	45	640
C phase	11	12	19	13	146
D phase	9	10	20	12	114
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
Ped 2	1				

Period: 8:00:00 PM to 8:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	36	169	71	711
C phase	10	12	19	13	139
D phase	5	3	12	10	50
Ped 2	3				

Period: 8:15:00 PM to 8:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	26	152	57	746
C phase	8	12	17	13	107
D phase	5	1	12	9	47
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 8:30:00 PM to 8:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	22	102	52	729
C phase	10	12	18	13	136
D phase	3	11	13	11	35
Nominal CL	2	28	60	44	

Active CL	2	28	60	44	
Ped 2	1				

Period: 8:45:00 PM to 9:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	33	110	86	781
C phase	4	12	16	14	56
D phase	5	11	14	12	63

Period: 9:00:00 PM to 9:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	42	162	78	781
C phase	7	12	18	13	97
D phase	2	11	11	11	22
Ped 2	1				

Period: 9:15:00 PM to 9:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	7	46	189	115	808
C phase	5	12	20	13	69
D phase	2	11	12	11	23
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 9:30:00 PM to 9:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	42	341	88	794
C phase	4	6	17	11	47
D phase	5	11	13	11	59
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 9:45:00 PM to 10:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	16	168	75	755
C phase	9	6	18	13	120
D phase	2	12	13	12	25
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
Ped 2	1				

Period: 10:00:00 PM to 10:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	42	165	86	775
C phase	6	7	18	14	87
D phase	3	11	14	12	38
Ped 2	1				

Period: 10:15:00 PM to 10:30:00 PM

Data	Freq.	Min	Мах	Avg	Total
A phase	7	11	225	116	814
C phase	6	6	16	12	73
D phase	1	13	13	13	13

Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 10:30:00 PM to 10:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	7	16	298	117	822
C phase	6	12	18	13	78
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
Ped 2	1				

Period: 10:45:00 PM to 11:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	4	35	716	216	865
D phase	3	11	12	11	35

Period: 11:00:00 PM to 11:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	8	30	213	102	819
C phase	4	12	12	12	48
D phase	3	11	11	11	33
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 11:15:00 PM to 11:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	6	6	240	139	834
C phase	4	12	18	13	54
D phase	1	12	12	12	12
Ped 2	1				

Period: 11:30:00 PM to 11:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 11:45:00 PM to Unknown

Data	Freq.	Min	Max	Avg	Total
? phase	1	5	5	5	5
A phase	2	118	764	441	882
C phase	1	12	12	12	12



С

SS=26



Report: Periodic statistics for site 708

15 minute intervals From 12:00:00 AM to 11:59:59 PM, on 15 August 2018

Period: 12:00:00 AM to 12:15:00 AM

Data	Freq.	Min	Max	Avg	Total
? phase	1	8	8	8	8
A phase	7	11	296	116	818
B phase	6	11	14	12	74

Period: 12:15:00 AM to 12:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	6	44	295	140	842
B phase	5	11	13	11	58

Period: 12:30:00 AM to 12:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	319	569	444	888
B phase	1	12	12	12	12

Period: 12:45:00 AM to 1:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	63	822	442	885
B phase	1	15	15	15	15

Period: 1:00:00 AM to 1:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	319	569	444	888
B phase	1	12	12	12	12

Period: 1:15:00 AM to 1:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	240	648	444	888
B phase	1	12	12	12	12

Period: 1:30:00 AM to 1:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	191	464	292	876
B phase	2	12	12	12	24

Period: 1:45:00 AM to 2:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:00:00 AM to 2:15:00 AM

Data	Freq.	Min	Max	Avg	Total		
A phase	1	900	900	900	900		

Data	Freq.	Min	Max	Avg	Total
A phase	2	345	543	444	888
B phase	1	12	12	12	12

Period: 2:30:00 AM to 2:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:45:00 AM to 3:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:00:00 AM to 3:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:15:00 AM to 3:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	416	473	444	889
B phase	1	11	11	11	11

Period: 3:30:00 AM to 3:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	12	587	290	870
B phase	1	11	11	11	11
C phase	1	19	19	19	19
IP3	1				
Ped 2	1				

Period: 3:45:00 AM to 4:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	268	618	443	886
B phase	1	14	14	14	14
IP2	1				
IP3	1				

Period: 4:00:00 AM to 4:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	39	624	292	876
B phase	2	12	12	12	24
IP2	2				
IP3	2				

Period: 4:15:00 AM to 4:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	180	512	291	875
B phase	2	12	13	12	25
IP2	3				
IP3	4				

Data	Freq.	Min	Max	Avg	Total
A phase	6	13	303	140	841
B phase	5	11	13	11	59
IP2	2				
IP3	3				

Period: 4:45:00 AM to 5:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	6	20	291	139	837
B phase	5	12	14	12	63
IP3	1				

Period: 5:00:00 AM to 5:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	111	613	292	878
B phase	2	11	11	11	22
IP3	1				

Period: 5:15:00 AM to 5:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	16	239	78	786
B phase	9	11	14	12	114
IP2	1				
IP3	1				

Period: 5:30:00 AM to 5:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	11	30	128	70	773
B phase	9	11	13	12	109
C phase	1	18	18	18	18
IP2	2				
IP3	1				
Ped 2	1				

Period: 5:45:00 AM to 6:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	13	24	157	55	724
B phase	12	11	19	13	157
C phase	1	19	19	19	19
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP2	4				
IP3	3				
Ped 2	1				

Period: 6:00:00 AM to 6:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	4	224	49	699
B phase	13	11	16	12	162
C phase	2	19	20	19	39
Nominal CL	3	28	60	49	
Active CL	3	28	60	49	
IP2	2				
IP3	2				

IP4	1		
Ped 2	2		

Period: 6:15:00 AM to 6:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	27	101	46	653
B phase	13	11	17	14	187
C phase	3	20	20	20	60
IP4	1				
IP5	1				
IP6	1				
IP7	2				
IP8	1				
Ped 2	3				

Period: 6:30:00 AM to 6:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	29	59	42	635
B phase	15	11	20	15	226
C phase	2	19	20	19	39
IP7	1				
IP8	1				
Ped 2	2				

Period: 6:45:00 AM to 7:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	12	63	46	647
B phase	13	12	21	14	193
C phase	3	20	20	20	60
Nominal CL	6	69	82	75	
Active CL	6	69	82	75	
IP7	2				
IP8	2				
Ped 2	3				

Period: 7:00:00 AM to 7:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	12	15	79	51	620
B phase	11	12	32	18	200
C phase	4	20	20	20	80
Nominal CL	10	75	90	84	
Active CL	10	75	90	84	
IP7	1				
IP8	1				
Ped 2	4				

Period: 7:15:00 AM to 7:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	50	83	68	681
B phase	9	13	31	22	199
C phase	1	20	20	20	20
Nominal CL	8	90	100	95	
Active CL	8	90	100	95	
IP8	1				
Ped 2	1				

Period: 7:30:00 AM to 7:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	22	98	62	622
B phase	9	15	26	19	178
C phase	5	20	20	20	100
Nominal CL	7	96	115	105	
Active CL	7	96	115	105	
IP6	2				
IP7	3				
IP8	2				
Ped 2	5				

Period: 7:45:00 AM to 8:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	39	91	65	659
B phase	9	15	24	20	181
C phase	3	20	20	20	60
Nominal CL	9	92	105	98	
Active CL	9	92	105	98	
IP5	1				
IP6	4				
IP7	2				
Ped 1	1				
Ped 2	3				

Period: 8:00:00 AM to 8:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	25	90	66	598
B phase	9	18	37	22	202
C phase	5	20	20	20	100
Nominal CL	8	100	112	106	
Active CL	8	100	112	106	
IP5	1				
IP6	2				
IP7	2				
IP8	1				
Ped 1	3				
Ped 2	5				

Period: 8:15:00 AM to 8:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	56	96	74	592
B phase	9	1	28	21	197
C phase	6	11	20	18	111
Nominal CL	8	105	120	112	
Active CL	8	105	120	112	
IP5	2				
IP6	2				
IP7	1				
Ped 1	2				
Ped 2	6				

Period: 8:30:00 AM to 8:45:00 AM

Data Freq. Min Max Avg Total

A phase	8	62	95	81	654
B phase	7	16	31	25	177
C phase	4	9	20	17	69
Nominal CL	4	117	120	118	
Active CL	4	117	120	118	
IP5	2				
IP6	2				
IP7	1				
Ped 1	2				
Ped 2	3				

Period: 8:45:00 AM to 9:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	21	100	69	628
B phase	9	3	26	19	172
C phase	5	20	20	20	100
Nominal CL	7	93	120	107	
Active CL	7	93	120	107	
IP6	2				
IP7	4				
IP8	1				
Ped 1	1				
Ped 2	5				

Period: 9:00:00 AM to 9:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	11	38	79	55	610
B phase	11	16	29	20	230
C phase	3	20	20	20	60
Nominal CL	11	62	97	81	
Active CL	11	62	97	81	
IP6	2				
IP7	4				
IP8	1				
Ped 1	1				
Ped 2	3				

Period: 9:15:00 AM to 9:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	13	6	65	43	570
B phase	13	12	28	17	230
C phase	5	20	20	20	100
Nominal CL	12	64	79	73	
Active CL	12	64	79	73	
IP6	1				
IP7	4				
IP8	2				
Ped 2	5				

Period: 9:30:00 AM to 9:45:00 AM

Data	Freq.	Min	Max	Avg	Total		
A phase	14	25	71	43	610		
B phase	14	12	30	17	251		
C phase	2	19	20	19	39		
Nominal CL	6	60	83	73			
Active CL	6	60	83	73			

IP5	1		
IP6	2		
IP7	3		
IP8	1		
Ped 2	2		

Period: 9:45:00 AM to 10:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	22	47	37	563
B phase	16	1	25	16	257
C phase	4	20	20	20	80
IP5	2				
IP6	3				
IP7	1				
Ped 2	4				

Period: 10:00:00 AM to 10:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	25	43	36	552
B phase	15	13	31	19	288
C phase	3	20	20	20	60
IP5	1				
IP6	4				
IP7	2				
Ped 2	3				

Period: 10:15:00 AM to 10:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	21	58	40	571
B phase	14	13	23	17	249
C phase	4	20	20	20	80
Nominal CL	8	60	76	70	
Active CL	8	60	76	70	
IP4	1				
IP5	4				
IP6	4				
IP7	1				
Ped 2	4				

Period: 10:30:00 AM to 10:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	8	60	35	568
B phase	15	12	26	16	253
C phase	4	19	20	19	79
IP5	2				
IP6	5				
IP7	4				
Ped 2	4				

Period: 10:45:00 AM to 11:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	4	56	34	550
B phase	15	11	29	19	290
C phase	3	20	20	20	60
Nominal CL	4	60	67	64	

Active CL	4	60	67	64	
IP4	1				
IP5	2				
IP6	1				
IP7	1				
Ped 1	1				
Ped 2	3				

Period: 11:00:00 AM to 11:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	3	49	36	585
B phase	15	11	24	17	255
C phase	3	20	20	20	60
IP5	3				
IP6	5				
IP7	2				
Ped 2	3				

Period: 11:15:00 AM to 11:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	6	46	36	589
B phase	15	14	34	19	291
C phase	1	20	20	20	20
Nominal CL	1	69	69	69	
Active CL	1	69	69	69	
IP5	4				
IP6	5				
IP7	1				
Ped 2	1				

Period: 11:30:00 AM to 11:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	29	55	42	633
B phase	14	13	27	19	267
Nominal CL	7	60	67	63	
Active CL	7	60	67	63	
IP4	1				
IP5	3				
IP6	3				
IP7	2				

Period: 11:45:00 AM to 12:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	11	48	34	550
B phase	15	12	39	20	310
C phase	2	20	20	20	40
IP3	1				
IP4	2				
IP5	4				
IP6	2				
Ped 1	1				
Ped 2	2				

Period: 12:00:00 PM to 12:15:00 PM

Data Freq. Min	Max	Avg	Total
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A phase	16	11	42	34	547
B phase	15	14	35	20	313
C phase	2	20	20	20	40
IP5	4				
IP6	4				
Ped 2	2				

Period: 12:15:00 PM to 12:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	13	58	39	589
B phase	14	12	26	17	251
C phase	3	20	20	20	60
Nominal CL	7	60	75	67	
Active CL	7	60	75	67	
IP4	1				
IP5	4				
IP6	4				
IP7	1				
Ped 1	1				
Ped 2	3				

Period: 12:30:00 PM to 12:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	6	49	34	556
B phase	15	12	28	19	285
C phase	3	19	20	19	59
IP5	3				
IP6	4				
IP7	2				
Ped 1	1				
Ped 2	3				

Period: 12:45:00 PM to 1:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	10	45	36	579
B phase	15	13	29	18	281
C phase	2	20	20	20	40
IP4	2				
IP5	5				
IP6	3				
IP7	1				
Ped 1	1				
Ped 2	2				

Period: 1:00:00 PM to 1:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	12	54	36	588
B phase	15	15	24	18	272
C phase	2	20	20	20	40
IP4	3				
IP5	5				
IP6	3				
IP7	1				
Ped 2	2				

Period: 1:15:00 PM to 1:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	8	46	35	562
B phase	15	12	35	18	279
C phase	3	19	20	19	59
IP5	3				
IP6	6				
IP7	2				
Ped 1	1				
Ped 2	3				

Period: 1:30:00 PM to 1:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	30	70	41	625
B phase	14	11	23	18	255
C phase	1	20	20	20	20
IP5	1				
IP6	3				
IP7	2				
Ped 1	1				
Ped 2	1				

Period: 1:45:00 PM to 2:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	8	46	34	559
B phase	15	14	25	18	281
C phase	3	20	20	20	60
IP3	2				
IP4	3				
IP5	4				
IP6	2				
Ped 2	3				

Period: 2:00:00 PM to 2:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	9	45	34	550
B phase	15	15	39	18	271
C phase	4	19	20	19	79
IP4	2				
IP5	2				
Ped 2	4				

Period: 2:15:00 PM to 2:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	6	62	38	584
B phase	14	14	31	21	296
C phase	1	20	20	20	20
Nominal CL	7	60	80	71	
Active CL	7	60	80	71	
IP4	3				
IP5	3				
Ped 2	1				

Period: 2:30:00 PM to 2:45:00 PM

Data	Free	Mim	Max	A.v.a	Total
Dala	I FIEQ.		Ινίαλ	AVY	IUlai

A phase	14	32	64	43	608
B phase	14	13	27	19	272
C phase	1	20	20	20	20
Nominal CL	5	67	82	74	
Active CL	5	67	82	74	
IP4	1				
IP5	3				
IP6	2				
IP7	1				
Ped 1	1				
Ped 2	1				

Period: 2:45:00 PM to 3:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	24	58	41	581
B phase	14	17	31	21	299
C phase	1	20	20	20	20
Nominal CL	6	60	74	67	
Active CL	6	60	74	67	
IP4	3				
IP5	7				
IP6	3				
Ped 2	1				

Period: 3:00:00 PM to 3:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	16	67	39	517
B phase	14	1	42	20	287
C phase	5	16	20	19	96
Nominal CL	7	60	94	77	
Active CL	7	60	94	77	
IP2	1				
IP3	4				
IP4	4				
IP5	2				
Ped 1	2				
Ped 2	5				

Period: 3:15:00 PM to 3:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	7	68	46	562
B phase	11	16	33	24	274
C phase	4	4	20	16	64
Nominal CL	8	74	92	82	
Active CL	8	74	92	82	
IP1	1				
IP2	1				
IP3	2				
IP4	1				
Ped 2	3				

Period: 3:30:00 PM to 3:45:00 PM

Data	Freq.	Min	Max	Avg	Total			
A phase	12	23	80	49	594			
B phase	12	1	34	23	286			
C phase	1	20	20	20	20			

Nominal CL	11	62	99	77	
Active CL	11	62	99	77	
IP1	2				
IP2	3				
IP3	3				
IP4	1				
Ped 2	1				

Period: 3:45:00 PM to 4:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	17	64	42	462
B phase	11	20	64	32	358
C phase	4	20	20	20	80
Nominal CL	6	60	105	94	
Active CL	6	60	105	94	
IP1	1				
IP2	1				
IP3	2				
IP4	1				
Ped 2	4				

Period: 4:00:00 PM to 4:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	27	76	57	572
B phase	10	18	61	26	268
C phase	3	20	20	20	60
Nominal CL	8	90	100	93	
Active CL	8	90	100	93	
IP1	1				
IP2	1				
Ped 1	1				
Ped 2	3				

Period: 4:15:00 PM to 4:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	34	82	55	559
B phase	10	15	40	26	261
C phase	4	20	20	20	80
Nominal CL	10	86	103	94	
Active CL	10	86	103	94	
IP1	1				
IP2	1				
Ped 2	4				

Period: 4:30:00 PM to 4:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	13	77	47	568
B phase	11	13	45	24	272
C phase	3	20	20	20	60
Nominal CL	9	71	96	82	
Active CL	9	71	96	82	
IP1	1				
IP2	2				
IP3	2				
IP4	1				
Ped 2	3				

Period: 4:45:00 PM to 5:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	17	67	38	494
B phase	12	16	45	27	326
C phase	4	20	20	20	80
Nominal CL	12	63	86	75	
Active CL	12	63	86	75	
IP2	1				
IP3	3				
IP4	3				
Ped 2	4				

Period: 5:00:00 PM to 5:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	31	74	56	560
B phase	9	17	40	26	240
C phase	5	20	20	20	100
Nominal CL	7	92	111	101	
Active CL	7	92	111	101	
IP1	1				
IP2	1				
IP3	1				
IP4	1				
Ped 1	1				
Ped 2	5				

Period: 5:15:00 PM to 5:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	31	74	50	509
B phase	9	14	64	36	331
C phase	3	20	20	20	60
Nominal CL	9	89	106	97	
Active CL	9	89	106	97	
IP1	2				
IP2	1				
Ped 1	2				
Ped 2	3				

Period: 5:30:00 PM to 5:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	8	13	98	54	435
B phase	8	18	82	48	390
C phase	4	15	20	18	75
Nominal CL	5	112	120	116	
Active CL	5	112	120	116	
IP1	1				
Ped 1	1				
Ped 2	4				

Period: 5:45:00 PM to 6:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	3	84	52	475
B phase	8	26	67	45	360
C phase	4	5	20	16	65

Nominal CL	8	98	120	111	
Active CL	8	98	120	111	
IP1	1				
Ped 1	1				
Ped 2	3				

Period: 6:00:00 PM to 6:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	29	70	51	621
B phase	12	4	39	21	259
C phase	1	20	20	20	20
Nominal CL	10	60	97	78	
Active CL	10	60	97	78	
IP1	3				
IP2	3				
IP3	1				
Ped 2	1				

Period: 6:15:00 PM to 6:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	22	72	47	568
B phase	13	8	30	22	292
C phase	2	20	20	20	40
Nominal CL	9	60	88	76	
Active CL	9	60	88	76	
IP2	1				
IP3	4				
IP4	3				
Ped 1	2				
Ped 2	2				

Period: 6:30:00 PM to 6:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	2	63	38	580
B phase	15	9	31	18	280
C phase	2	20	20	20	40
Nominal CL	3	70	77	74	
Active CL	3	70	77	74	
IP2	1				
IP3	1				
IP4	1				
IP5	1				
IP6	2				
IP7	1				
Ped 2	2				

Period: 6:45:00 PM to 7:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	22	63	44	620
B phase	14	10	26	18	260
C phase	1	20	20	20	20
Nominal CL	9	60	78	68	
Active CL	9	60	78	68	
IP4	1				
IP5	4				
IP6	4				

IP7	1		
Ped 2	1		

Period: 7:00:00 PM to 7:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	23	58	44	624
B phase	14	9	21	16	236
C phase	2	20	20	20	40
Nominal CL	8	60	75	68	
Active CL	8	60	75	68	
IP5	3				
IP6	4				
IP7	2				
Ped 2	2				

Period: 7:15:00 PM to 7:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	6	48	38	613
B phase	14	14	24	17	247
C phase	2	20	20	20	40
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
IP5	3				
IP6	3				
IP7	1				
Ped 2	2				

Period: 7:30:00 PM to 7:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	6	104	43	648
B phase	15	8	21	15	232
C phase	1	20	20	20	20
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
IP4	2				
IP5	4				
IP6	2				
Ped 1	2				
Ped 2	1				

Period: 7:45:00 PM to 8:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	24	71	43	655
B phase	15	8	31	16	245
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
IP2	1				
IP3	4				
IP4	4				

Period: 8:00:00 PM to 8:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	14	67	42	678
B phase	15	12	19	14	222
IP4	1				

IP5	2		
IP6	4		
IP7	2		

Period: 8:15:00 PM to 8:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	25	60	41	662
B phase	15	12	23	15	238
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	2				
IP2	2				
IP3	1				
IP4	1				
IP5	1				
IP6	1				

Period: 8:30:00 PM to 8:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	17	8	50	38	654
B phase	16	12	21	15	246
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	3				
IP2	4				
IP3	1				

Period: 8:45:00 PM to 9:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	3	229	67	738
B phase	10	12	21	16	162
IP1	1				
IP2	4				
IP3	3				
IP4	1				

Period: 9:00:00 PM to 9:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	5	97	46	692
B phase	14	12	25	14	208
IP1	1				
IP2	3				
IP3	2				
IP4	1				

Period: 9:15:00 PM to 9:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	17	1	72	40	683
B phase	16	11	20	13	217
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	2				
IP2	3				
IP3	2				
IP4	1				

Period: 9:30:00 PM to 9:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	7	106	43	690
B phase	15	12	21	14	210
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP2	2				
IP3	2				

Period: 9:45:00 PM to 10:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	18	13	50	34	629
B phase	17	11	28	14	251
C phase	1	20	20	20	20
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	1				
IP2	3				
IP3	1				
Ped 2	1				

Period: 10:00:00 PM to 10:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	31	194	62	746
B phase	11	11	18	14	154
IP2	3				
IP3	4				
IP4	2				
IP5	1				

Period: 10:15:00 PM to 10:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	1	158	64	770
B phase	11	11	14	11	130
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	1				
IP2	2				
IP3	1				
IP4	1				
IP5	1				

Period: 10:30:00 PM to 10:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	11	261	79	791
B phase	9	11	13	12	109
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
IP1	3				
IP2	5				
IP3	1				

Data	Freq.	Min	Max	Avg	Total
A phase	10	12	155	78	785
B phase	9	11	17	12	115
IP1	5				
IP2	5				

Period: 11:00:00 PM to 11:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	6	352	89	801
B phase	8	11	15	12	99
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	4				
IP2	3				

Period: 11:15:00 PM to 11:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	7	15	374	118	827
B phase	6	11	13	12	73
IP1	3				
IP2	2				

Period: 11:30:00 PM to 11:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	5	4	407	169	848
B phase	4	11	16	13	52
IP1	3				
IP2	3				

Period: 11:45:00 PM to Unknown

Data	Freq.	Min	Max	Avg	Total
? phase	1	11	11	11	11
A phase	7	20	297	116	812
B phase	6	11	17	12	76
IP1	2				
IP2	2				

APPENDIX D

SIDRA MOVEMENT SUMMARIES

SITE LAYOUT

Site: 101 [LON_RIVX AM]

Longueville Rd, River Rd & Northwood Rd, Lane Cove Signals - Fixed Time Isolated



Site: 101 [LON_RIVX AM]

♦♦ Network: N101 [Existing AM (Existing Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove

Signals - Fixed Time Coordinated Cycle Time = 112 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles												
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective /	Average
ID	Mov	Iotal	ΗV	Iotal	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: North	wood Rd (S)										
1a	L1	611	0.8	611	0.8	0.367	2.6	LOS A	1.5	10.6	0.05	0.49	45.7
2	T1	288	1.0	288	1.0	1.030	106.9	LOS F	6.9	49.0	1.00	1.27	1.3
Appro	ach	899	0.9	899	0.9	1.030	36.0	LOS C	6.9	49.0	0.35	0.74	21.2
North:	Longu	eville Rd (N	I)										
8	T1	264	0.8	264	0.8	0.957	86.9	LOS F	7.4	52.2	1.00	1.19	1.8
Appro	ach	264	0.8	264	0.8	0.957	86.9	LOS F	7.4	52.2	1.00	1.19	1.8
North\	West: R	liver Rd We	est (NW	/)									
27b	L3	179	6.1	179	6.1	0.991	83.8	LOS F	69.4	491.8	1.00	1.23	15.6
29a	R1	1293	0.1	1293	0.1	0.991	82.7	LOS F	69.4	491.8	1.00	1.23	15.4
Appro	ach	1472	0.8	1472	0.8	0.991	82.9	LOS F	69.4	491.8	1.00	1.23	15.4
All Ve	hicles	2635	0.8	2635	0.8	1.030	67.3	LOS E	69.4	491.8	0.78	1.06	15.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 32.5 % Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
	Decemption	ped/h	sec	Service	pedesthan	m	Queuea	per ped						
P3	North Full Crossing	6	5.5	LOS A	0.0	0.0	0.31	0.31						
P7	NorthWest Full Crossing	17	50.2	LOS E	0.1	0.1	0.95	0.95						
All Pe	destrians	23	38.5	LOS D			0.78	0.78						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: Z:\DATA\Data\Jobs01\Jobs\17work\17311_266LonguevilleRdLaneCove\SIDRA\180824\Existing Volumes (Existing Layout).sip7

Site: 101 [LON_RIVX PM]

Network: N101 [Existing PM (Existing Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove

Signals - Fixed Time Coordinated Cycle Time = 87 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles												
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	Average
ID	Mov	Iotal	ΗV	Iotal	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: North	vood Rd (S	5)										
1a	L1	1267	0.1	1267	0.1	0.630	7.8	LOS A	6.9	49.0	0.21	0.56	40.9
2	T1	289	1.4	289	1.4	0.630	32.4	LOS C	6.9	49.0	1.00	0.88	4.1
Appro	ach	1556	0.3	1556	0.3	0.630	12.4	LOS A	6.9	49.0	0.36	0.62	35.4
North:	Longu	eville Rd (N	I)										
8	T1	324	0.9	324	0.9	0.279	17.1	LOS B	5.7	39.9	0.68	0.57	7.8
Appro	ach	324	0.9	324	0.9	0.279	17.1	LOS B	5.7	39.9	0.68	0.57	7.8
North\	Nest: R	liver Rd We	est (NW	/)									
27b	L3	84	3.6	84	3.6	0.629	27.0	LOS B	14.6	103.0	0.84	0.84	29.6
29a	R1	614	0.5	614	0.5	0.629	25.3	LOS B	14.6	103.0	0.84	0.83	29.6
Appro	ach	698	0.9	698	0.9	0.629	25.5	LOS B	14.6	103.0	0.84	0.83	29.6
All Ve	hicles	2578	0.5	2578	0.5	0.630	16.5	LOS B	14.6	103.0	0.53	0.67	31.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 19.6 % Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
	Description	ped/h	belay sec	Service	pedesinan	Distance	Queuea	per ped						
P3	North Full Crossing	3	18.7	LOS B	0.0	0.0	0.66	0.66						
P7	NorthWest Full Crossing	10	37.7	LOS D	0.0	0.0	0.93	0.93						
All Pe	destrians	13	33.3	LOS D			0.87	0.87						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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SITE LAYOUT

Site: 101 [LON_RIVP(P) AM]

Longueville Rd, River Rd & Northwood Rd, Lane Cove Signals - Fixed Time Isolated



Site: 101 [LON_RIVP(P) AM]

♦♦ Network: N101 [Proposed AM (Proposed Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove

Signals - Fixed Time Coordinated Cycle Time = 114 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles												
Mov	OD	Demand I	Flows	Arriva	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective /	Average
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: North	wood Rd (S))										
1a	L1	621	0.8	621	0.8	0.384	2.9	LOS A	2.1	15.0	0.07	0.50	45.5
2	T1	288	1.0	288	1.0	0.839	62.4	LOS E	6.9	49.0	1.00	0.92	2.3
Appro	ach	909	0.9	909	0.9	0.839	21.7	LOS B	6.9	49.0	0.36	0.63	27.5
North:	Longu	eville Rd (N)										
8	T1	261	0.8	261	0.8	0.477	36.7	LOS C	7.4	52.2	0.88	0.74	4.0
9b	R3	43	0.0	43	0.0	0.511	65.3	LOS E	2.5	17.7	1.00	0.75	19.6
Appro	ach	304	0.7	304	0.7	0.511	40.8	LOS C	7.4	52.2	0.90	0.75	8.5
North\	West: R	liver Rd We	st (NW	/)									
27b	L3	179	6.1	179	6.1	0.861	28.5	LOS C	47.2	334.0	0.89	0.91	28.9
29a	R1	1293	0.1	1293	0.1	0.861	31.1	LOS C	47.2	334.0	0.89	0.93	27.1
Appro	ach	1472	0.8	1472	0.8	0.861	30.8	LOS C	47.2	334.0	0.89	0.92	27.3
All Vel	hicles	2685	0.8	2685	0.8	0.861	28.8	LOS C	47.2	334.0	0.71	0.80	25.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 5.0 % Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov	5	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m		per ped						
P3	North Full Crossing	6	11.4	LOS B	0.0	0.0	0.45	0.45						
P7	NorthWest Full Crossing	17	51.2	LOS E	0.1	0.1	0.95	0.95						
All Pe	destrians	23	40.8	LOS E			0.82	0.82						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [LON_RIVP(P) PM]

•• Network: N101 [Proposed PM (Proposed Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove

Signals - Fixed Time Coordinated Cycle Time = 88 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles												
Mov	OD	Demand I	Flows	Arriva	l Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective .	Average
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: North	vood Rd (S))										
1a	L1	1279	0.1	1279	0.1	0.691	12.8	LOS A	7.0	49.0	0.64	0.76	37.0
2	T1	289	1.4	289	1.4	0.691	37.4	LOS C	6.9	49.0	1.00	0.88	3.6
Appro	ach	1568	0.3	1568	0.3	0.691	17.4	LOS B	7.0	49.0	0.71	0.78	32.0
North:	Longu	eville Rd (N)										
8	T1	318	0.9	318	0.9	0.319	13.4	LOS A	7.4	52.2	0.62	0.54	9.5
9b	R3	45	0.0	45	0.0	0.413	49.7	LOS D	2.0	14.1	1.00	0.74	23.2
Appro	ach	363	0.8	363	0.8	0.413	17.9	LOS B	7.4	52.2	0.67	0.56	15.3
North\	West: R	iver Rd We	st (NW	/)									
27b	L3	84	3.6	84	3.6	0.711	32.5	LOS C	17.5	123.6	0.92	0.85	27.2
29a	R1	614	0.5	614	0.5	0.711	31.9	LOS C	17.5	123.6	0.92	0.87	26.7
Appro	ach	698	0.9	698	0.9	0.711	32.0	LOS C	17.5	123.6	0.92	0.86	26.8
All Ve	hicles	2629	0.5	2629	0.5	0.711	21.3	LOS B	17.5	123.6	0.76	0.77	29.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 145.2 % Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
UI ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m		per ped						
P3	North Full Crossing	3	23.3	LOS C	0.0	0.0	0.73	0.73						
P7	NorthWest Full Crossing	10	38.2	LOS D	0.0	0.0	0.93	0.93						
All Pe	destrians	13	34.8	LOS D			0.88	0.88						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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SITE LAYOUT

Site: 101 [NOR_KENX AM]

Northwood Rd & Kenneth St Signals - Fixed Time Isolated



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Site: 101 [NOR_KENX AM]

♦♦ Network: N101 [Existing AM (Existing Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 112 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles													
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed	
												Rate		
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Kenne	th St (S)												
1a	L1	164	1.2	164	1.2	0.196	24.1	LOS B	5.4	38.5	0.65	0.70	30.2	
3b	R3	104	1.9	104	1.9	0.922	78.4	LOS F	6.9	49.0	1.00	1.04	23.9	
Appro	ach	268	1.5	268	1.5	0.922	45.2	LOS D	6.9	49.0	0.79	0.83	26.4	
South	East: No	orthwood F	Rd (SE))										
21b	L3	23	4.3	23	4.3	0.597	29.8	LOS C	21.3	150.0	0.80	0.72	37.3	
22	T1	761	0.8	761	0.8	0.597	24.6	LOS B	21.3	150.0	0.80	0.71	29.9	
Appro	ach	784	0.9	784	0.9	0.597	24.7	LOS B	21.3	150.0	0.80	0.71	30.2	
North\	West: N	orthwood F	Rd (NW	/)										
28	T1	1473	0.1	1473	0.1	0.521	3.4	LOS A	7.0	49.0	0.33	0.31	46.0	
29a	R1	161	0.6	161	0.6	0.786	42.6	LOS D	7.0	49.0	0.84	0.88	23.9	
Appro	ach	1634	0.2	1634	0.2	0.786	7.3	LOS A	7.0	49.0	0.38	0.36	42.1	
All Ve	hicles	2686	0.5	2686	0.5	0.922	16.1	LOS B	21.3	150.0	0.55	0.51	35.5	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 32.5 % Number of Iterations: 10 (maximum specified: 10)

Movement Performance - Pedestrians												
Mov	Description	Demand	Average	Level of a	Average Back	of Queue	Prop.	Effective				
ט ו		ped/h	sec	Service	pedestnan	Distance	Queuea	per ped				
P1	South Full Crossing	14	20.7	LOS C	0.0	0.0	0.61	0.61				
P5	SouthEast Full Crossing	12	50.2	LOS E	0.0	0.0	0.95	0.95				
All Pe	destrians	26	34.3	LOS D			0.76	0.76				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [NOR_KENX PM]

Network: N101 [Existing PM (Existing Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 87 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles												
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed
												Rate	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Kenne	th St (S)											
1a	L1	168	2.4	168	2.4	0.275	27.3	LOS B	5.3	38.1	0.79	0.75	28.7
3b	R3	71	2.8	71	2.8	0.786	56.4	LOS D	3.4	24.6	1.00	0.91	27.9
Appro	ach	239	2.5	239	2.5	0.786	35.9	LOS C	5.3	38.1	0.85	0.79	28.4
South	East: No	orthwood R	d (SE))									
21b	L3	21	0.0	21	0.0	0.870	31.7	LOS C	39.4	276.1	0.94	0.96	36.6
22	T1	1346	0.1	1346	0.1	0.870	29.5	LOS C	39.4	276.1	0.94	1.00	27.7
Appro	ach	1367	0.1	1367	0.1	0.870	29.5	LOS C	39.4	276.1	0.94	1.00	27.9
North\	West: N	orthwood F	Rd (NW	/)									
28	T1	730	0.5	730	0.5	0.241	2.3	LOS A	3.9	27.3	0.26	0.23	47.2
29a	R1	194	0.5	194	0.5	0.895	51.1	LOS D	7.0	49.0	1.00	0.98	21.7
Appro	ach	924	0.5	924	0.5	0.895	12.6	LOS A	7.0	49.0	0.42	0.39	37.8
All Ve	hicles	2530	0.5	2530	0.5	0.895	23.9	LOS B	39.4	276.1	0.74	0.75	30.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 19.6 % Number of Iterations: 10 (maximum specified: 10)

Movement Performance - Pedestrians												
Mov	Description	Demand	Average	Level of a	Average Back	of Queue	Prop.	Effective Stop Poto				
שו	Decemption	ped/h	sec	Service	pedestnan	m	Queuea	per ped				
P1	South Full Crossing	2	12.7	LOS B	0.0	0.0	0.54	0.54				
P5	SouthEast Full Crossing	28	37.8	LOS D	0.1	0.1	0.93	0.93				
All Pe	destrians	30	36.1	LOS D			0.91	0.91				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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SITE LAYOUT

Site: 101 [NOR_KENP(P) AM]

Northwood Rd & Kenneth St Signals - Fixed Time Isolated



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Site: 101 [NOR_KENP(P) AM]

+ Network: N101 [Proposed AM (Proposed Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 114 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles												
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed
												Rate	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Kenne	th St (S)											
1a	L1	164	1.2	164	1.2	0.174	20.6	LOS B	5.0	35.3	0.59	0.68	32.1
3b	R3	104	1.9	104	1.9	0.938	82.6	LOS F	7.2	50.9	1.00	1.06	23.3
Appro	ach	268	1.5	268	1.5	0.938	44.7	LOS D	7.2	50.9	0.75	0.83	26.5
South	East: No	orthwood F	Rd (SE))									
21b	L3	23	4.3	23	4.3	0.682	35.4	LOS C	24.2	170.7	0.88	0.79	35.2
22	T1	771	0.8	771	0.8	0.682	30.5	LOS C	24.2	170.7	0.88	0.79	27.3
Appro	ach	794	0.9	794	0.9	0.682	30.6	LOS C	24.2	170.7	0.88	0.79	27.6
North\	West: N	orthwood F	Rd (NW	/)									
28	T1	1486	0.1	1486	0.1	0.514	3.1	LOS A	7.0	49.0	0.31	0.29	46.3
29a	R1	145	0.7	145	0.7	0.682	39.2	LOS C	7.0	49.0	0.92	0.80	25.0
Appro	ach	1631	0.2	1631	0.2	0.682	6.3	LOS A	7.0	49.0	0.37	0.33	43.0
All Ve	hicles	2693	0.5	2693	0.5	0.938	17.3	LOS B	24.2	170.7	0.56	0.51	34.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 5.0 % Number of Iterations: 10 (maximum specified: 10)

Movement Performance - Pedestrians												
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
שו	Description	ped/h	sec	Service	pedesinan	Distance	Queuea	per ped				
P1	South Full Crossing	14	24.7	LOS C	0.0	0.0	0.66	0.66				
P5	SouthEast Full Crossing	12	51.2	LOS E	0.0	0.0	0.95	0.95				
All Pe	destrians	26	36.9	LOS D			0.79	0.79				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [NOR_KENP(P) PM]

♦ Network: N101 [Proposed PM (Proposed Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 88 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles												
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed
												Rate	
		veh/h	%	veh/h	%	V/C	sec		veh	m		per veh	km/h
South	: Kenne	th St (S)											
1a	L1	168	2.4	168	2.4	0.288	28.7	LOS C	5.5	39.4	0.80	0.75	28.1
3b	R3	71	2.8	71	2.8	0.795	57.3	LOS E	3.5	25.0	1.00	0.91	27.8
Appro	ach	239	2.5	239	2.5	0.795	37.2	LOS C	5.5	39.4	0.86	0.80	28.0
South	East: N	orthwood F	Rd (SE))									
21b	L3	21	0.0	21	0.0	0.852	28.3	LOS B	37.5	262.7	0.91	0.91	37.9
22	T1	1358	0.1	1358	0.1	0.852	25.8	LOS B	37.5	262.7	0.91	0.94	29.3
Appro	ach	1379	0.1	1379	0.1	0.852	25.9	LOS B	37.5	262.7	0.91	0.94	29.5
North\	West: N	orthwood F	Rd (NW	/)									
28	T1	741	0.5	741	0.5	0.243	1.0	LOS A	1.9	13.5	0.10	0.09	48.7
29a	R1	175	0.6	175	0.6	0.866	50.3	LOS D	7.0	49.0	1.00	0.94	21.9
Appro	ach	916	0.5	916	0.5	0.866	10.4	LOS A	7.0	49.0	0.27	0.25	39.4
All Ve	hicles	2534	0.5	2534	0.5	0.866	21.4	LOS B	37.5	262.7	0.68	0.68	32.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 145.2 % Number of Iterations: 10 (maximum specified: 10)

Movement Performance - Pedestrians												
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
שו	Description	ped/h	Delay sec	Service	pedesinan ped	Distance	Queuea	per ped				
P1	South Full Crossing	2	12.0	LOS B	0.0	0.0	0.52	0.52				
P5	SouthEast Full Crossing	28	38.2	LOS D	0.1	0.1	0.93	0.93				
All Pe	destrians	30	36.5	LOS D			0.91	0.91				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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SITE LAYOUT

∇ Site: 101 [LON_ACCX AM]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)



V Site: 101 [LON_ACCX AM]

♦ Network: N101 [Existing AM (Existing Arrangements)]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Longu	ievile Rd (S)										
2	T1	460	3.0	460	3.0	0.245	0.0	LOS A	0.0	0.1	0.00	0.00	50.0
3	R2	1	0.0	1	0.0	0.245	4.5	LOS A	0.0	0.1	0.00	0.00	21.5
Appro	ach	461	3.0	461	3.0	0.245	0.0	NA	0.0	0.1	0.00	0.00	49.8
East:	Site Aco	cess Drivew	/ay (E)										
4	L2	3	0.0	3	0.0	0.012	0.4	LOS A	0.0	0.2	0.33	0.22	19.4
6	R2	3	0.0	3	0.0	0.012	5.2	LOS A	0.0	0.2	0.33	0.22	27.6
Appro	ach	6	0.0	6	0.0	0.012	2.8	LOS A	0.0	0.2	0.33	0.22	24.2
North:	Longu	evile Rd (N))										
7	L2	1	0.0	1	0.0	0.071	4.6	LOS A	0.4	2.8	0.00	0.00	49.5
8	T1	270	1.9	270	1.9	0.071	0.0	LOS A	0.4	2.8	0.00	0.00	50.0
Appro	ach	271	1.8	271	1.8	0.071	0.0	NA	0.4	2.8	0.00	0.00	50.0
All Ve	hicles	738	2.6	738	2.6	0.245	0.0	NA	0.4	2.8	0.00	0.00	49.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 32.5 % Number of Iterations: 10 (maximum specified: 10)

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V Site: 101 [LON_ACCX PM]

Network: N101 [Existing PM (Existing Arrangements)]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Longı	uevile Rd (S)										
2	T1	369	1.9	369	1.9	0.197	0.0	LOS A	0.0	0.1	0.00	0.00	49.9
3	R2	2	0.0	2	0.0	0.197	4.8	LOS A	0.0	0.1	0.00	0.00	21.5
Appro	ach	371	1.9	371	1.9	0.197	0.0	NA	0.0	0.1	0.00	0.00	49.6
East:	Site Ac	cess Drivew	vay (E)										
4	L2	1	0.0	1	0.0	0.005	0.4	LOS A	0.0	0.1	0.40	0.26	19.3
6	R2	2	0.0	2	0.0	0.005	4.7	LOS A	0.0	0.1	0.40	0.26	27.5
Appro	ach	3	0.0	3	0.0	0.005	3.3	LOS A	0.0	0.1	0.40	0.26	25.3
North:	Longu	evile Rd (N)										
7	L2	3	0.0	3	0.0	0.094	4.6	LOS A	0.0	0.0	0.00	0.01	49.4
8	T1	326	1.8	326	1.8	0.094	0.0	LOS A	0.0	0.0	0.00	0.00	49.9
Appro	ach	329	1.8	329	1.8	0.094	0.1	NA	0.0	0.0	0.00	0.01	49.9
All Ve	hicles	703	1.8	703	1.8	0.197	0.1	NA	0.0	0.1	0.00	0.01	49.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 19.6 % Number of Iterations: 10 (maximum specified: 10)

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SITE LAYOUT

Site: 101 [LON_ACCP(P) AM]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)



V Site: 101 [LON_ACCP(P) AM]

♦ Network: N101 [Proposed AM (Proposed Arrangements)]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand F Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Longu	evile Rd (S))										
2	T1	460	3.0	460	3.0	0.244	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Appro	ach	460	3.0	460	3.0	0.244	0.0	NA	0.0	0.0	0.00	0.00	50.0
East:	Site Acc	ess Drivew	ay (E)										
4	L2	37	0.0	37	0.0	0.025	0.3	LOS A	0.1	0.7	0.21	0.08	19.8
Appro	ach	37	0.0	37	0.0	0.025	0.3	LOS A	0.1	0.7	0.21	0.08	19.8
North:	Longue	evile Rd (N)											
7	L2	30	0.0	30	0.0	0.079	4.6	LOS A	3.6	25.3	0.00	0.11	48.9
8	T1	270	1.9	270	1.9	0.079	0.0	LOS A	3.6	25.3	0.00	0.05	49.4
Appro	ach	300	1.7	300	1.7	0.079	0.5	NA	3.6	25.3	0.00	0.05	49.3
All Ve	hicles	797	2.4	797	2.4	0.244	0.2	NA	3.6	25.3	0.01	0.02	46.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 5.0 % Number of Iterations: 10 (maximum specified: 10)

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V Site: 101 [LON_ACCP(P) PM]

♦ Network: N101 [Proposed PM (Proposed Arrangements)]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand F Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop S Rate	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Longu	evile Rd (S))										
2	T1	369	1.9	369	1.9	0.195	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Appro	ach	369	1.9	369	1.9	0.195	0.0	NA	0.0	0.0	0.00	0.00	50.0
East:	Site Acc	ess Drivew	ay (E)										
4	L2	33	0.0	33	0.0	0.023	0.4	LOS A	0.1	0.6	0.23	0.10	19.8
Appro	ach	33	0.0	33	0.0	0.023	0.4	LOS A	0.1	0.6	0.23	0.10	19.8
North:	Longue	evile Rd (N)											
7	L2	36	0.0	36	0.0	0.095	4.6	LOS A	0.4	2.9	0.00	0.11	48.9
8	T1	326	1.8	326	1.8	0.095	0.0	LOS A	0.4	2.9	0.00	0.05	49.4
Appro	ach	362	1.7	362	1.7	0.095	0.5	NA	0.4	2.9	0.00	0.05	49.3
All Ve	hicles	764	1.7	764	1.7	0.195	0.2	NA	0.4	2.9	0.01	0.03	46.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 145.2 % Number of Iterations: 10 (maximum specified: 10)

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